



February 8, 2016

Project #: 19056

Kerry Martin SOZO Sports of Central Washington 1200 Chesterley Drive, Suite 140 Yakima, WA 98902

RE: Transportation Impact Analysis for Yakima Sports Complex – Supplemental Analysis

Dear Kerry,

This letter summarizes new transportation analyses for the Yakima Sports Complex and is intended to supplement our original October 13, 2015 *Transportation Impact Analysis for the Yakima Sports Complex.*

Background

The October 13, 2015 Transportation Impact Analysis (TIA) was completed assuming full buildout of the 58-acre sports complex (accommodating 13 outdoor soccer fields and an 80,000 square foot indoor facility with the ability to house up to six multi-use athletic courts/fields). The analysis concluded that full buildout of the project would require the following off-site improvements:

- Signalization of the S. 38th Avenue/Ahtanum Road intersection, including the widening of Ahtanum Road for an eastbound left-turn lane and an exclusive westbound right-turn lane. Based on an iterations analysis, it was concluded that signalization and associated widening of Ahtanum Road would be needed at the equivalent of 12 or more soccer fields.
- Construction of a second exclusive lane (for right-turning vehicles) on the S. 48th Avenue (from Spring Creek Road to W. Washington Avenue) approach to the W. Washington Avenue/S. 48th Avenue intersection. It was further concluded that the second lane should be constructed as part of the initial development of the soccer complex in order to better accommodate the existing and projected right-turn movement and minimize delays to the predominate right-turn maneuver.

Following the completion of the transportation impact analysis, new development details have raised additional circulation questions as it pertains to the overall phasing of the soccer complex and how it will be impacted by the City of Yakima's plans to reconstruct/widen Spring Creek Road and S. 36th Avenue. To address these questions, the following three phasing scenarios have been analyzed:

- 1. Phase 1 Partial 8-field build-out of the sports complex prior to reconstruction of Spring Creek Road and S. 36th Avenue.
- 2. Phase 2 Partial 8-field build-out of the sports complex during reconstruction of Spring Creek Road and S. 36th Avenue.
- 3. Phase 3 Full build-out of the entire sports complex after reconstruction of Spring Creek Road and S. 36th Avenue.

PHASE 1 – PARTIAL 8-FIELD BUILDOUT PRIOR TO SPRING CREEK ROAD/S. 36TH **AVENUE RECONSTRUCTION**

In order to better accommodate circulation to/from the soccer complex, the City of Yakima is considering the reconstruction and widening of Spring Creek Road and S. 36th Avenue between W. Washington Avenue and Sorenson Road. In recognition that this reconstruction/widening project likely won't be completed until sometime in 2017, an interim Phase 1 analysis was completed to identify the near-term transportation impacts of a partial buildout of the soccer complex. This analysis was prepared under the following assumptions:

- Spring Creek Road and S. 36th Avenue are not improved beyond existing conditions.
- Eight soccer fields are constructed and in use as early as Spring 2016.
- SOZO Sports will construct an extension of Sorenson Road west of the S. 38th Avenue/Sorenson Road intersection along their southerly site frontage. As part of this extension, the S. 38th Avenue/Sorenson Road intersection will be reconfigured to a more traditional three-legged intersection.
- In the near-term, vehicular access to the 8-field soccer complex will be via a temporary driveway located off of the westerly extension of Sorenson Road approximately 250 feet west of the S. 38th Avenue/Sorenson Road intersection. Figure 1 illustrates the Phase 1 site plan.
- All other study area intersections and roadways will remain unchanged.
- All other study area mice.

 8 soccer fields will generate approximately 142 weekuay.

 Saturday midday peak hour trips as documented in Table 1. These trip generation estimates have been developed consistent with the methodology outlined in the October 2015 TIARECEIVED

 FEB 0 8 2016 8 soccer fields will generate approximately 142 weekday PM peak hour trips and 243

¹ Based on conversations with SOZO Sports, they would ideally like to have a total of eight soccer fields ready for use/play in 2016.

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Figure 1 – Phase 1 Site Plan (Prepared by Digital Design & Development)

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YAKIMA, WASHINGTON

SOZO SPORTS, INC PROPOSED SPORTS COMPLEX

Design Development SITE PLAN

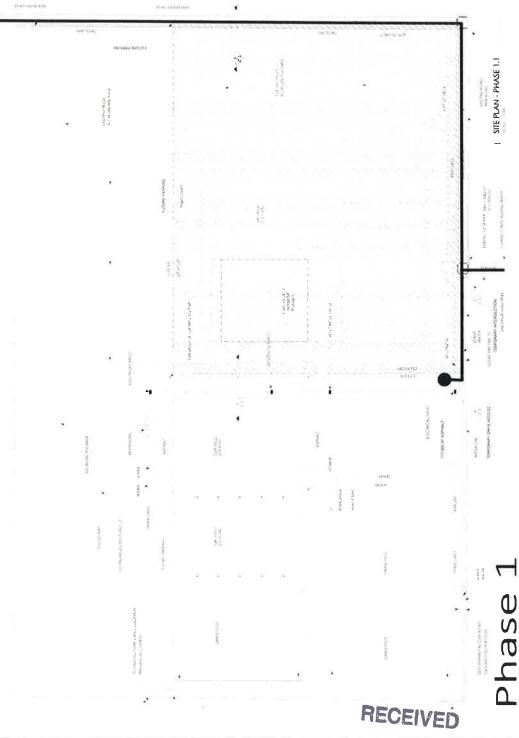


Table 1 - Proposed Yakima Sports Complex Trip Generation (Partial Build-out)

	ITE Code	C:	Weekday	Weel	day PM Peak	Hour	Saturday	Peak Hour of	Generator
Land Use	ITE Code	Size	Daily Trips	Total	In	Out	Total	In	Out
Soccer Complex	488	8 Fields	571	142	95	47	243	117	126

Under these development assumptions, the following questions have been addressed:

- 1. What are the operational impacts of 8 soccer fields at the S 38th Avenue/Ahtanum Road intersection?
- 2. What are the operation impacts of 8 soccer fields at the W. Washington Avenue/S. 48th Avenue intersection?
- 3. What are the operational impacts of 8 soccer fields to each of the local streets along Spring Creek Road?
- 4. What are the operational impacts of 8 soccer fields at the reconstructed S. 38th
 Avenue/Sorenson Road intersection, and what form of traffic control mitigation is necessary to accommodate buildout of Phase 1?

1. S. 38th Avenue/Ahtanum Road Intersection Operations

The October 13, 2015 TIA identified the need for mitigation at the S. 38th Avenue/Ahtanum Road intersection as it related to full buildout of the soccer complex. Utilizing the same distribution assumptions identified in the October 13, 2015 TIA, a supplemental analysis of this intersection under the partial 8 field build-out determined the following:

- The critical southbound approach (S. 38th Avenue) is forecast to operate at an acceptable level of service (LOS) C during both the weekday PM and Saturday midday peak hours.
- The intersection is not forecast to meet traffic signal warrants in accordance with the procedures outlined in the 2009 Manual on Uniform Traffic Control Devices (MUTCD).

Appendix A contains the Phase 1 operations worksheets for the S. 38th Avenue/Ahtanum Road intersection.

2. S. 48th Avenue/W. Washington Avenue Intersection

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A supplemental operations and queuing analysis was performed at the S. 48th Avenue/W. Washington Avenue intersection under the partial 8-field build-out and determined the following:

The critical northbound approach (S. 48th Avenue) is expected to operate at an acceptable level of service (LOS) B during both the weekday PM and Saturday midday peak hours.

A 95th percentile queuing analysis on the northbound S. 48th Avenue approach to W. Washington Avenue determined the queues for 8 fields can be managed under the existing single-lane approach. However, as described in the October 2015 TIA, the additional traffic from the soccer complex will add new right-turn demand from S. 48th Avenue onto W. Washington Avenue. Without a separate right-turn lane, there will likely be instances of long vehicle queues when traffic volumes spike at the end of games/practices/events. Therefore, it is recommended that the right-turn lane be constructed as soon as possible, but no later than the corresponding reconstruction of Spring Creek Road/S. 36th Avenue.

Appendix A contains the Phase 1 operations worksheets for the S. 48th Avenue/W. Washington Avenue intersection.

3. Spring Creek Road Delay Analysis

In order to address traffic concerns raised by the adjacent residential neighborhoods to the north of the soccer complex, a delay analysis was performed at each of the local streets (S. 40th Avenue, S. 41st Avenue, S. 44th Avenue, S. 45th Avenue, and S. 47th Avenue) that access Spring Creek Road between S. 48th Avenue and S. 36th Avenue under the partial 8-field build-out. To perform this analysis, traffic volumes on Spring Creek Road at each of these five intersections were estimated based on traffic volumes collected at the 48th Avenue/W. Washington Avenue intersection. Additionally, turning movement volumes to/from each side street were estimated based on trip generation estimates (number of residential units served by each local street) obtained from ITE's Trip Generation. Table 2 summarizes the resulting delay analysis at these intersections.

Table 2 - Spring Creek Road Delay Analysis - Partial 8-Field Build-out

	2 A L	Spring Cr	eek / 40th	Spring Cr	eek / 41st	Spring Cr	eek / 44th	Spring Cr	eek / 45th	Spring Cr	eek / 47th
Number of Houses	Served		6	1	.6	2	25		4	2	:3
Time Period		PM	SAT								
Trin Consusting	In	4	3	10	8	16	13	3	2	14	12
Trip Generation	Out	2	3	6	7	9	11	1	2	9	10
Northbound LOS (8	fields)	Α	Α	Α	Α	Α	Α	Α	Α	Α	A

As shown in the table, the critical northbound approach at each of the five local streets are forecast to operate with minimal delay (LOS A) during both the weekday PM and Saturday midday peak hours with 8 soccer fields in operation. Appendix A contains the Phase 1 operations worksheets for the Spring 1 Poad intersections.

S. 38th Avenue and Sorenson Road currently link together through a horizontal curve at a location where there are no turn movements. Upon development of the soccer complex, the horizontal curve will be reconstructed to form a three-legged intersection (new west leg providing temporary access to

Kittelson & Associates, Inc. Portland, Oregon the soccer complex). With the exception of late afternoons and evenings at the start and end of sporting events, future traffic volumes through the intersection to/from the partial 8-field build-out will be minimal. Based on the anticipated traffic pattern, implementation of yield control is recommended on the northbound approach to the reconstructed S. 38th Avenue/Sorenson Road intersection.

Provision of yield control on the northbound approach will allow off-peak/non-event traffic to traverse the predominate north-to-east maneuver with minimal delay. Further, implementation of northbound approach yield control will provide adequate intersection capacity when east-west volumes through the intersection increase during periods of peak traffic to and from the soccer complex. By way of comparison, all-way stop control implementation is not recommended because all-way stop control warrants will not be met and drivers might begin to disregard stop signs in a setting where they observe no other traffic during off-peak periods. Appendix A contains the Phase 1 operations worksheets for the S. 38th Avenue/Sorenson Road intersection.



SCENARIO 2 – PARTIAL 8-FIELD BUILDOUT DURING SPRING CREEK ROAD/S. 36TH AVENUE RECONSTRUCTION

As previously mentioned the City of Yakima is considering the reconstruction and widening of Spring Creek Road and S. 36th Avenue between W. Washington Avenue and Sorenson Road. Although the design details are still being analyzed, it is anticipated that this reconstruction/widening will include a three-lane cross section with sidewalks and bike lanes. For the purposes of this analysis, completion is preliminarily assumed for 2017. During the reconstruction process, it is anticipated that both Spring Creek Road and S. 36th Avenue will need to be closed off to all non-local through traffic, including the traffic generated by the soccer complex. With this assumed restriction in place, a supplemental Phase 2 analysis was completed to identify the near-term transportation impacts of the partial 8-field soccer complex remaining open. This analysis was completed under the following assumptions:

- Spring Creek Road and S. 36th Avenue are closed off and all site-generated trips to/from the 8-field soccer complex are forced to access the site via Ahtanum Road and S. 38th Avenue.
- Vehicular site access off of the extension of Sorenson Road will remain unchanged as shown in Figure 2.
- All other study area intersections and roadways will remain unchanged.

Under these development assumptions, the following questions have been addressed:

- 1. What are the operational impacts of 8 soccer fields at the S. 38th Avenue/Ahtanum Road intersection under the condition that this intersection is accommodating all of the soccer complex traffic?
- 2. What are the operational impacts of 8 soccer fields at the reconstructed S. 38th Avenue/Sorenson Road intersection?



Figure 2 – Phase 2 Site Plan (Prepared by Digital Design & Development)



1. S. 38th Avenue/Ahtanum Road Intersection

Utilizing the same trip generation assumptions identified in Phase 1 and assuming that Spring Creek Road and S. 36th Avenue are under reconstruction and closed to all through traffic, a supplemental analysis of this intersection under the partial 8 field soccer complex determined the following:

- The critical southbound approach (S. 38th Avenue) is expected to operate at LOS F during the weekday PM peak hour.
- The intersection is forecast to meet traffic signal warrants.
 - With signalization, the intersection is forecast to return to acceptable levels of service. This assumes construction of an eastbound left-turn lane and protected leftturn phasing. Although not necessary to address level of service or capacity constraints, a westbound right-turn lane on Ahtanum Road would provide additional operations and safety benefits.

Appendix B contains the Phase 2 operations worksheets for the S. 38th Avenue/Ahtanum Road intersection.

2. S. 38th Avenue/Sorenson Road

As previously noted, Sorenson Road will be extended west of the S. 38th Avenue/Sorenson Road intersection. In addition, the existing S. 38th Avenue/Sorenson Road intersection will be reconfigured to a more traditional three-legged intersection. With these changes, the critical approach (northbound S. 38th Avenue) is expected to operate at an acceptable LOS B or better during both the weekday PM and Saturday midday peak hours. Appendix B contains the Phase 2 operations worksheets for the S. 38th Avenue/Sorenson Road intersection.

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SCENARIO 3 – FULL BUILD-OUT AFTER SPRING CREEK ROAD RECONSTRUCTION

At the time the October 2015 TIA was prepared, it was unclear what level of improvements the City of Yakima was prepared to make along Spring Creek Road and S. 36th Avenue. With the previously noted reconstruction and widening improvements now under formal consideration, a supplemental analysis has been prepared to address the full-build transportation impacts associated with this widening. This analysis was prepared under the following assumptions:

- Spring Creek Road and S. 36th Avenue are reconstructed and fully improved to a three-lane cross section.
- The 58-acre sports complex will be fully built out as documented in the October 2015 TIA.
- A new permanent site access driveway will be constructed off of S. 36th Avenue. The driveway is assumed to include separate left- and right-turn exiting lanes onto S. 36th Avenue while S. 36th Avenue is assumed to include a northbound left-turn lane. Figure 3 displays the full build-out site plan under Scenario #3.
- The temporary site access off of the westerly extension of Sorenson Road will be closed and converted to an emergency access. A secondary site access driveway will be constructed to form the north leg of the S. 38th Avenue/Sorenson Road intersection. It is anticipated that this driveway will be closed off most of the time and open only during special events.

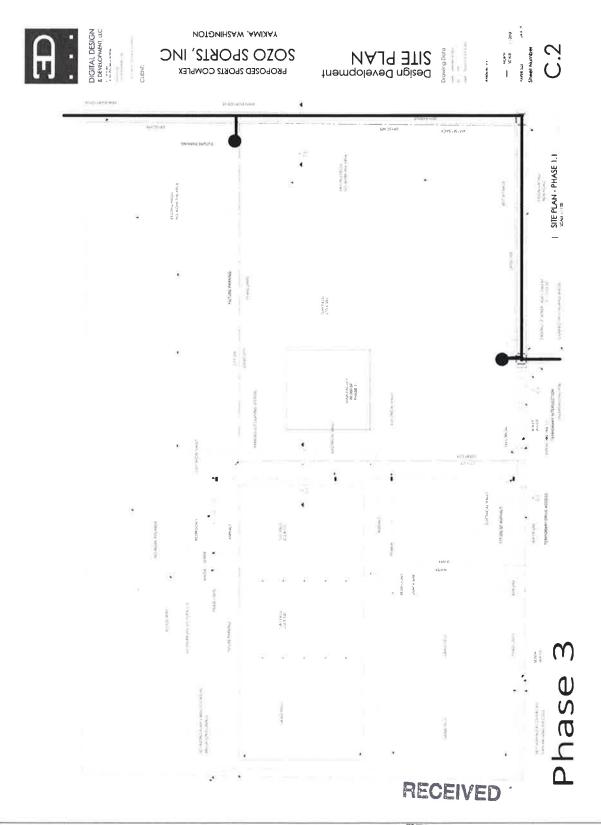
Under these development assumptions, the following questions have been addressed:

- 1. What are the operational impacts of full build-out of the soccer complex to each of the local streets along Spring Creek Road (assuming the widening of Spring Creek Road)?
- 2. What are the operational impacts of full build-out of the soccer complex at the proposed main site access off of S. 36th Avenue?
- 3. What is the recommended geometric configuration at the reconstructed S. 36th Avenue/Sorenson Road intersection?
- 4. What are the longer-term recommendations at the S. 38th Avenue/Ahtanum Road intersection?

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Figure 3 – Phase 3 Full Build Out Site Plan (Prepared by Digital Design & Development)



1. Spring Creek Road Delay Analysis

In order to address traffic concerns raised by the adjacent residential neighborhoods to the north of the soccer complex, a delay analysis was performed at each of the local streets (S. 40th Avenue, S. 41st Avenue, S. 44th Avenue, S. 45th Avenue, and S. 47th Avenue) that access Spring Creek Road between S. 48th Avenue and S. 36th Avenue under full build-out of the proposed sports complex. As described under Phase 1, traffic volumes on Spring Creek Road at each of these five intersections were estimated based on traffic volumes collected at the 48th Avenue/W. Washington Avenue intersection. Additionally, turning movement volumes to/from each side street were estimated based on trip generation estimates (number of residential units served by each local street) obtained from ITE's Trip Generation. Table 3 summarizes the resulting delay analysis at these intersections.

Intersection Spring Creek / 40th Spring Creek / 41st Spring Creek / 44th Spring Creek / 45th Spring Creek / 47th 25 Houses 16 23 Time Period PM SAT PM SAT PM SAT PM SAT PM SAT 3 3 2 13 12 Trip Gen 2 7 2 Out 3 6 9 1 9 11 10 Northbound LOS (full-build)

Table 3 - Spring Creek Road Delay Analysis - Full Site Build-out

As shown in the table, the critical northbound approach at each of the five local streets are forecast to operate with minimal delay (LOS A) during both the weekday PM and Saturday midday peak hours after full build-out of the proposed sports complex and reconstruction of Spring Creek Road. Appendix C contains the Phase 1 operations worksheets for the Spring Creek Road intersections.

2. Site Access Operations (S. 36th Avenue)

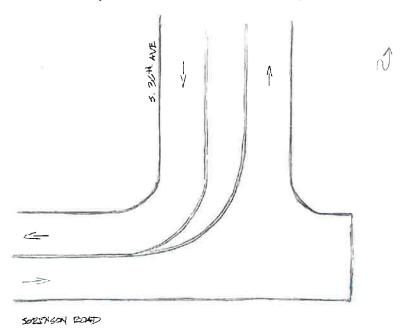
A detailed operations analysis was performed at the proposed main site access off of S. 36th Avenue. Assuming the site access layout that was previously outlined, the critical eastbound left-turn is forecast to operate at LOS B during both the weekday PM peak hour and Saturday midday peak hours after full build-out of the proposed sports complex. Appendix C contains the Phase 3 operations worksheets for the permanent site access driveway on S. 36th Avenue.

3. S. 36th Avenue/Sorenson Road Intersection

S. 36th Avenue and Sorenson Road currently link together through a horizontal curve at a location where there are no turn movements. Upon reconstruction of S. 36th Avenue, it is assumed that the horizontal curve will be reconstructed to form a traditional 90 degree intersection (with a new street stub on the east leg to accommodate a potential easterly extension of Sorenson Road). Until the easterly extension of Sorenson is established, it is recommend that the intersection be left uncontrolled and striped according to the conceptual sketch shown in Exhibit 1 below.

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Exhibit 1 - Conceptual Sketch of the S. 36th Avenue/Sorenson Road Intersection



4. S. 38th Avenue/Ahtanum Road Intersection

As documented in the October 13, 2015 Transportation Impact Analysis (TIA), it was concluded that signalization and widening of the S. 38th Avenue/Ahtanum Road intersection would be needed at the equivalent of 12 or more soccer fields. Given that signalization and widening of the intersection will be needed during the Phase 2 reconstruction of Spring Creek Road and S. 36th Avenue, it is recommended that this signalization remain following Phase 2. At some point in the future if/when an easterly extension of Sorenson Road connects to Ahtanum Road (via a new north/south roadway), the need for continued signalization of the S. 38th Avenue/Ahtanum Road intersection can be revisited by Yakima County.

We trust this document adequately addresses the supplemental questions regarding the traffic impacts associated with the proposed Yakima Sports Complex. Please contact us if you have any questions.

Sincerely,

KITTELSON & ASSOCIATES, INC.

Matt Hughart, AICP Associate Planner

Math Kuytan

Julia Kuhn, PE Principal Engineer FEB 0 8 2016 CITY OF YAKIMA PLANNING DIV

Appendix A Phase 1 Operations Worksheets

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Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		4	P	Mileseyita	M	NO SHIP A
Traffic Volume (veh/h)	13	353	542	100	86	22
Future Volume (Veh/h)	13	353	542	100	86	22
Sign Control		Free	Free		Stop	
Grade		0%	0%		0%	
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Hourly flow rate (vph)	13	353	542	100	86	22
Pedestrians		000	0.2			
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type		None	None			
Median storage veh)		None	110110			
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	642				971	592
vC1, stage 1 conf vol	042				371	33Z
vC2, stage 2 conf vol						
vCu, unblocked vol	642				971	592
tC, single (s)	4.2				6.4	6.2
	4.2				0.4	Ų.Z
tC, 2 stage (s)	2.3				3.5	3.3
tF (s)	2.3 99				69	96
p0 queue free %					276	510
cM capacity (veh/h)	914	Land Water Design			210	טוט
Direction, Lane #	EB 1	W8 1	SB 1			
Volume Total	366	642	108			
Volume Left	13	0	86			
Volume Right	0	100	22			
cSH	914	1700	305			
Volume to Capacity	0.01	0.38	0.35			
Queue Length 95th (ft)	1	0	39			
Control Delay (s)	0.5	0.0	23.1			
Lane LOS	Α		С			
Approach Delay (s)	0.5	0.0	23.1			
Approach LOS			С			
Intersection Summary					400	A 64 =
Average Delay			2.4			
Intersection Capacity Utiliz	ation		47.4%	IC	U Level o	f Service
Analysis Period (min)			15			
marysis r chou (min)			10			

	-	*	•	-	4	-	
Movement	EBT	EBR	WEL	WBT	NBL	NBR	
Lane Configurations	13) releasible	Mauso	4	M	T. Carrott	
Traffic Volume (veh/h)	23	23	65	47	47	26	
Future Volume (Veh/h)	23	23	65	47	47	26	
Sign Control	Free			Free	Yield	20	
Grade	0%			0%	0%		
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Hourly flow rate (vph)	23	23	65	47	47	26	
Pedestrians			00		- "	20	
Lane Width (ft)							
Walking Speed (ft/s)							
Percent Blockage							
Right turn flare (veh)							
Median type	None			None			
Median storage veh)				110110			
Upstream signal (ft)							
pX, platoon unblocked							
vC, conflicting volume			46		212	34	
vC1, stage 1 conf vol						0.	
vC2, stage 2 conf vol							
vCu, unblocked vol			46		212	34	
tC, single (s)			4.1		6.4	6.2	
tC, 2 stage (s)						VI.E	
tF (s)			2.2		3.5	3.3	
p0 queue free %			96		94	97	
cM capacity (veh/h)			1562		744	1039	
Direction, Lane #	EB 1	WB.1	NB 1				
Volume Total	46	112	73				
Volume Left	0	65	47				
Volume Right	23	00	26				
cSH	1700	1562	828				
Volume to Capacity	0.03	0.04	0.09				
Queue Length 95th (ft)	0.03	3	7				
Control Delay (s)	0.0	4.4	9.8				
Lane LOS	0.0						
Approach Delay (s)	0.0	A 4.4	A 9.8				
Approach LOS	0.0	4.4	9.8 A				
			А				
Intersection Summary						- L	
Average Delay			5.2				
Intersection Capacity Utiliza	ation		23.6%	IC	U Level o	f Service	
Analysis Period (min)			15				

	→	•	•	←	4	~
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	fa fa			4	N	
Traffic Volume (veh/h)	99	7	7	114	5	4
Future Volume (Veh/h)	99	7	7	114	5	4
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	108	8	8	124	5	4
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	None			None		
Median storage veh)	. 101,0					
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume			116		252	112
vC1, stage 1 conf vol			.,,			
vC2, stage 2 conf vol						
vCu, unblocked vol			116		252	112
tC, single (s)			4.1		6.4	6.2
tC, 2 stage (s)			4.1		0.4	0.2
tF (s)			2.2		3.5	3.3
p0 queue free %			99		99	100
			1485		737	947
cM capacity (veh/h)					131	947
Direction, Lane #	EB 1	WB 1	NB 1			
Volume Total	116	132	9			
Volume Left	0	8	5			
Volume Right	8	0	4			
cSH	1700	1485	817			
Volume to Capacity	0.07	0.01	0.01			
Queue Length 95th (ft)	0	0	1			
Control Delay (s)	0.0	0.5	9.5			
Lane LOS		Α	Α			
Approach Delay (s)	0.0	0.5	9.5			
Approach LOS			Α			
Intersection Summary					W 13	ا د ال
Average Delay			0.6			
Intersection Capacity Utiliz	zation		21.7%	IC	U Level o	f Service
Analysis Period (min)	Lation		15	10	C 201010	50, 1100
Analysis i ellou (IIIII)			10			

	-	•	•	4-	1	~
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	1.			ન	W	
Traffic Volume (veh/h)	101	2	1	120	1	0
Future Volume (Veh/h)	101	2	1	120	1	0
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	110	2	1	130	1	0
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	None			None		
Median storage veh)	. 10110			1,0110		
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume			112		243	111
vC1, stage 1 conf vol			112		470	- 111
vC2, stage 2 conf voi						
vCu, unblocked vol			112		243	111
tC, single (s)			4.1		6.4	6.2
tC, 2 stage (s)			4.1		0.4	0.2
tF (s)			2.2		3.5	3.3
p0 queue free %			100		100	100
cM capacity (veh/h)			1490		749	948
					149	948
Direction, Lane #	EB 1	WB 1	NB 1			
Volume Total	112	131	1			
Volume Left	0	1	1			
Volume Right	2	0	0			
cSH	1700	1490	749			
Volume to Capacity	0.07	0.00	0.00			
Queue Length 95th (ft)	0	0	0			
Control Delay (s)	0.0	0.1	9.8			
Lane LOS		Α	Α			
Approach Delay (s)	0.0	0.1	9.8			
Approach LOS			Α			
Intersection Summary						
Average Delay			0.1			
Intersection Capacity Utiliza	ation		17.1%	IC	U Level o	f Service
Analysis Period (min)			15		2 23 701 0	. 551 1105
maryona i criod (iliili)			10			



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Movement	EBT	EBR	WBL	WBT	NBL	NBR	
Lane Configurations	ĵ _a			र्स	W		
Traffic Volume (veh/h)	93	8	8	116	5	4	
Future Volume (Veh/h)	93	8	8	116	5	4	
Sign Control	Free			Free	Stop		
Grade	0%			0%	0%		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	
Hourly flow rate (vph)	101	9	9	126	5	4	
Pedestrians	101	Ů	Ů	120			
Lane Width (ft)							
Walking Speed (ft/s)							
Percent Blockage							
Right turn flare (veh)							
Median type	None			None			
	None			NOUG			
Median storage veh) Upstream signal (ft)							
pX, platoon unblocked			440		050	400	
vC, conflicting volume			110		250	106	
vC1, stage 1 conf vol							
vC2, stage 2 conf vol						100	
vCu, unblocked vol			110		250	106	
tC, single (s)			4.1		6.4	6.2	
tC, 2 stage (s)							
tF (s)			2.2		3.5	3.3	
p0 queue free %			99		99	100	
cM capacity (veh/h)			1493		739	954	
Direction, Lane #	EB 1	WB 1	NB 1				
Volume Total	110	135	9				
Volume Left	0	9	5				
Volume Right	9	0	4				
cSH	1700	1493	821				
Volume to Capacity	0.06	0.01	0.01				
Queue Length 95th (ft)	0	0	1				
Control Delay (s)	0.0	0.5	9.4				
Lane LOS		A	Α				
Approach Delay (s)	0.0	0.5	9.4				
Approach LOS	0.0	310	A				
Intersection Summary				017.1	12.75	elfo.	
Average Delay			0.6				
Intersection Capacity Utiliza	ation		22.7%	ıc	U Level o	of Service	Α
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	→	*	1	←	1	~	
Movement	EBT	EBR	WBL	WBT	NBL	NBR	11,48,500
Lane Configurations	1			सं	N.		
Traffic Volume (veh/h)	92	5	5	128	6	5	
Future Volume (Veh/h)	92	5	5	128	6	5	
Sign Control	Free			Free	Stop		
Grade	0%			0%	0%		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	
Hourly flow rate (vph)	100	5	5	139	7	5	
Pedestrians							
Lane Width (ft)							
Walking Speed (ft/s)							
Percent Blockage							
Right turn flare (veh)							
Median type	None			None			
Median storage veh)							
Upstream signal (ft)							
pX, platoon unblocked							
vC, conflicting volume			105		252	102	
vC1, stage 1 conf vol							
vC2, stage 2 conf vol							
vCu, unblocked vol			105		252	102	
tC, single (s)			4.1		6.4	6.2	
tC, 2 stage (s)					0.1	0.2	
tF (s)			2.2		3.5	3.3	
p0 queue free %			100		99	99	
cM capacity (veh/h)			1499		739	958	
Direction, Lane #	EB 1	WB 1	NB 1		100	000	
Volume Total	105	144	12				
Volume Left	0	5	7				
Volume Right	5	0	5				
cSH	1700	1499	817				
Volume to Capacity	0.06	0.00	0.01				
Queue Length 95th (ft)	0.0	0.3	9.5				
Control Delay (s)	0.0						
Lane LOS	0.0	A	A				
Approach Delay (s)	0.0	0.3	9.5				
Approach LOS			Α				
Intersection Summary	Jan e	1.00					
Average Delay			0.6				
Intersection Capacity Utiliza	tion		20.8%	IC	U Level o	f Service	Α
Analysis Period (min)			15				

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Movement	EBT	EBR	WBL	WBT	NBL	NBR	
Lane Configurations	4			ની	W		
Traffic Volume (veh/h)	95	2	2	122	1	1	
Future Volume (Veh/h)	95	2	2	122	1	1	
Sign Control	Free			Free	Stop		
Grade	0%			0%	0%		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	
Hourly flow rate (vph)	103	2	2	133	1	1	
Pedestrians							
Lane Width (ft)							
Walking Speed (ft/s)							
Percent Blockage							
Right turn flare (veh)							
Median type	None			None			
Median storage veh)	-						
Upstream signal (ft)							
pX, platoon unblocked							
vC, conflicting volume			105		241	104	
vC1, stage 1 conf vol							
vC2, stage 2 conf vol							
vCu, unblocked vol			105		241	104	
tC, single (s)			4.1		6.4	6.2	
tC, 2 stage (s)					0.1	V.L	
tF (s)			2.2		3.5	3.3	
p0 queue free %			100		100	100	
cM capacity (veh/h)			1499		751	956	
	PP V	7.8.05.3			701	000	
Direction, Lane #	EB 1	WB 1	NB 1				
Volume Total	105	135	2				
Volume Left	0	2	1				
Volume Right	2	0	1				
cSH	1700	1499	841				
Volume to Capacity	0.06	0.00	0.00				
Queue Length 95th (ft)	0	0	0				
Control Delay (s)	0.0	0.1	9.3				
Lane LOS		Α	Α				
Approach Delay (s)	0.0	0.1	9.3				
Approach LOS			Α				
Intersection Summary	¥11.		di za		W 1		
Average Delay			0.1				
Intersection Capacity Utilizat	ion		18.0%	IC	U Level o	of Service	Α
Analysis Period (min)			15				

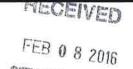
	•	•	†	-	-	Ţ	
Movement	WBL	WBR	NBT	NBR	SBL	SBT	
Lane Configurations	A		1		7	11	
Traffic Volume (veh/h)	9	111	570	34	71	900	
Future Volume (Veh/h)	9	111	570	34	71	900	
Sign Control	Stop		Free			Free	
Grade	0%		0%			0%	
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Hourly flow rate (vph)	9	111	570	34	71	900	
Pedestrians	1						
Lane Width (ft)	12.0						
Walking Speed (ft/s)	4.0						
Percent Blockage	0						
Right turn flare (veh)	·						
Median type			None			None	
Median storage veh)			110110			140110	
Jpstream signal (ft)							
oX, platoon unblocked							
C, conflicting volume	1180	303			605		
/C1, stage 1 conf vol	1100	303			000		
C2, stage 2 conf vol							
/Cu, unblocked vol	1180	303			605		
	6.8	6.9					
C, single (s)	0.0	0.9			4.2		
C, 2 stage (s)	2.5	2.0			0.0		
tF (s)	3.5	3.3			2.2		
p0 queue free %	95	84			93		
cM capacity (veh/h)	172	698			955		
Direction, Lane #	WB 1	NB 1	NB 2	SB 1	SB 2	SB 3	
/olume Total	120	380	224	71	450	450	
Volume Left	9	0	0	71	0	0	
Volume Right	111	0	34	0	0	0	
cSH	568	1700	1700	955	1700	1700	
Volume to Capacity	0.21	0.22	0.13	0.07	0.26	0.26	
Queue Length 95th (ft)	20	0	0	6	0	0	
Control Delay (s)	13.0	0.0	0.0	9.1	0.0	0.0	
Lane LOS	В			Α			
Approach Delay (s)	13.0	0.0		0.7			
Approach LOS	В						
Intersection Summary					1000		
Average Delay			1.3				
Intersection Capacity Utiliza	ation		38.9%	IC	U Level	of Service	Α
Analysis Period (min)			15				



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Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		स	f)		M	Harris
Traffic Volume (veh/h)	18	418	276	72	73	18
Future Volume (Veh/h)	18	418	276	72	73	18
Sign Control		Free	Free		Stop	
Grade		0%	0%		0%	
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Hourly flow rate (vph)	18	418	276	72	73	18
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type		None	None			
Median storage veh)		110110	110110			
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	348				766	312
vC1, stage 1 conf vol	010				100	012
vC2, stage 2 conf vol						
vCu, unblocked vol	348				766	312
tC, single (s)	4.2				6.4	6.2
tC, 2 stage (s)	7.2				0.4	0.2
tF (s)	2.3				3.5	3.3
p0 queue free %	98				80	98
cM capacity (veh/h)	1178				365	733
		VAVE CA	05.4		303	733
Direction, Lane #	EB 1	WB 1	SB 1 91	20		
Volume Total	436	348				
Volume Left	18	0	73			
Volume Right	0	72	18			
cSH	1178	1700	405			
Volume to Capacity	0.02	0.20	0.22			
Queue Length 95th (ft)	1	0	21			
Control Delay (s)	0.5	0.0	16.4			
Lane LOS	Α		С			
Approach Delay (s)	0.5	0.0	16.4			
Approach LOS			С			
Intersection Summary			-1-			150
Average Delay			2.0			
Intersection Capacity Utiliza	ation		48.4%	IC	U Level o	f Service
Analysis Period (min)			15			

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Movement	EBT	EBR	WBL	WBT	NBL	NBR	ng na
Lane Configurations	î.			र्न	N		
Traffic Volume (veh/h)	63	63	62	58	58	39	
Future Volume (Veh/h)	63	63	62	58	58	39	
Sign Control	Free			Free	Yield		
Grade	0%			0%	0%		
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Hourly flow rate (vph)	63	63	62	58	58	39	
Pedestrians							
Lane Width (ft)							
Walking Speed (ft/s)							
Percent Blockage							
Right turn flare (veh)							
Median type	None			None			
Median storage veh)	110110			110110			
Upstream signal (ft)							
pX, platoon unblocked							
vC, conflicting volume			126		276	94	
vC1, stage 1 conf vol			120		210		
vC2, stage 2 conf vol							
vCu, unblocked vol			126		276	94	
tC, single (s)			4.1		6.4	6.2	
tC, 2 stage (s)			7,1		0.4	0.2	
tF (s)			2.2		3.5	3.3	
p0 queue free %			96		92	96	
cM capacity (veh/h)			1460		683	962	
					003	902	
Direction, Lane #	EB 1	WB 1	NB 1				
Volume Total	126	120	97				
Volume Left	0	62	58				
Volume Right	63	0	39				
cSH	1700	1460	773				
Volume to Capacity	0.07	0.04	0.13				
Queue Length 95th (ft)	0	3	11				
Control Delay (s)	0.0	4.1	10.3				
Lane LOS		Α	В				
Approach Delay (s)	0.0	4.1	10.3				
Approach LOS			В				
Intersection Summary			TALL.		-		
Average Delay			4.3				
Intersection Capacity Utiliza	ation		29.3%	IC	ll l evel d	of Service	
Analysis Period (min)	uu OII		15	10	O LOVEI C	VI OCIAICE	
Aliaiyaia Feliuu (Illiii)			10				

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Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	ĵ.		H. Carrier L.	ની	M	
Traffic Volume (veh/h)	74	6	6	91	5	5
Future Volume (Veh/h)	74	6	6	91	5	5
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	80	7	7	99	5	5
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	None			None		
Median storage veh)						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume			87		196	84
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol			87		196	84
tC, single (s)			4.1		6.4	6.2
tC, 2 stage (s)			7.1		0.7	V.E
tF (s)			2.2		3.5	3.3
p0 queue free %			100		99	99
cM capacity (veh/h)			1522		793	981
Direction, Lane #	EB 1	WB 1	NB 1		, 00	
	87	106	10			
Volume Total						
Volume Left	0	7	5			
Volume Right	7	0	5			
cSH	1700	1522	877			
Volume to Capacity	0.05	0.00	0.01			
Queue Length 95th (ft)	0	0	1			
Control Delay (s)	0.0	0.5	9.2			
Lane LOS		Α.	A			
Approach Delay (s)	0.0	0.5	9.2			
Approach LOS			Α			
Intersection Summary			- 11		- 34	
Average Delay			0.7			
Intersection Capacity Utiliza	ation		19.7%	IC	U Level o	of Service
Analysis Period (min)			15			
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Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	\$			न	N.	
Traffic Volume (veh/h)	78	1	1	96	1	1
Future Volume (Veh/h)	78	1	1	96	1	1
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	85	1	1	104	1	1
Pedestrians					-	
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	None			None		
Median storage veh)	110110			110110		
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume			86		192	86
vC1, stage 1 conf vol			00		192	00
vC2, stage 2 conf vol						
vCu, unblocked vol			86		192	86
tC, single (s)			4.1		6.4	6.2
			4.1		0.4	0.2
tC, 2 stage (s)			2.2		2.5	2.2
tF (s)					3.5	3.3
p0 queue free %			100		100	100
cM capacity (veh/h)			1523		801	979
Direction, Lane #	EB 1	WB 1	NB 1			11.00
Volume Total	86	105	2			
Volume Left	0	1	1			
Volume Right	1	0	1			
cSH	1700	1523	881			
Volume to Capacity	0.05	0.00	0.00			
Queue Length 95th (ft)	0	0	0			
Control Delay (s)	0.0	0.1	9.1			
Lane LOS		Α	Α			
Approach Delay (s)	0.0	0.1	9.1			
Approach LOS			Α			
Intersection Summary						
Average Delay			0.1			
Intersection Capacity Utiliz	ation		15.8%	IC	U Level o	f Service
Analysis Period (min)			15.578	10	- LO VOI C	
Analysis i enou (IIIII)			IJ			

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Movement	EBT	EBR	WBL	WBT	NBL	NBR	
Lane Configurations	7	0,000		र्स	M		
Traffic Volume (veh/h)	82	7	6	89	6	5	
Future Volume (Veh/h)	82	7	6	89	6	5	
Sign Control	Free			Free	Stop		
Grade	0%			0%	0%		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	
Hourly flow rate (vph)	89	8	7	97	7	5	
Pedestrians	00	- Č	•	- 07			
Lane Width (ft)							
Walking Speed (ft/s)							
Percent Blockage							
Right turn flare (veh)							
Median type	None			None			
Median type Median storage veh)	None			None			
Jpstream signal (ft)							
oX, platoon unblocked			07		004	00	
C, conflicting volume			97		204	93	
/C1, stage 1 conf vol							
vC2, stage 2 conf vol							
/Cu, unblocked vol			97		204	93	
C, single (s)			4.1		6.4	6.2	
tC, 2 stage (s)							
F (s)			2.2		3.5	3.3	
00 queue free %			100		99	99	
cM capacity (veh/h)			1509		785	970	
irection, Lane #	EB 1	WB 1	NB 1	8	11.00	N IE N	
/olume Total	97	104	12				
/olume Left	0	7	7				
/olume Right	8	0	5				
SH	1700	1509	853				
/olume to Capacity	0.06	0.00	0.01				
Queue Length 95th (ft)	0	0	1				
Control Delay (s)	0.0	0.5	9.3				
ane LOS	0.0	A	Α.				
Approach Delay (s)	0.0	0.5	9.3				
Approach LOS	0.0	0.0	3.5 A				
						- XII	Control of the Contro
Intersection Summary Average Delay			0.8	1	1		MANERY PROPERTY NAMED IN CO.
Intersection Capacity Utiliza	ation		19.6%	IC	U Level o	f Service	A
Analysis Period (min)	auun		15.0%	iC	O LEVEL	I OCI VICE	^
Analysis Period (Min)			10				

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Movement	EST	EBR	WBL	WBT	NBL	NER	
Lane Configurations	4	THE PARTY OF THE P		र्स	W		
Traffic Volume (veh/h)	83	4	4	95	4	3	
Future Volume (Veh/h)	83	4	4	95	4	3	
Sign Control	Free			Free	Stop		
Grade	0%			0%	0%		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	
Hourly flow rate (vph)	90	4	4	103	4	3	
Pedestrians							
Lane Width (ft)							
Walking Speed (ft/s)							
Percent Blockage							
Right turn flare (veh)							
Median type	None			None			
Median storage veh)							
Upstream signal (ft)							
pX, platoon unblocked							
vC, conflicting volume			94		203	92	
vC1, stage 1 conf vol							
vC2, stage 2 conf vol							
vCu, unblocked vol			94		203	92	
tC, single (s)			4.1		6.4	6.2	
tC, 2 stage (s)			- 111				
tF(s)			2.2		3.5	3.3	
p0 queue free %			100		99	100	
cM capacity (veh/h)			1513		788	971	
Direction, Lane #	EB 1	WB 1	NB 1				
Volume Total	94	107	7				
Volume Left	0	4	4				
Volume Right	4	0	3				
cSH	1700	1513	857				
Volume to Capacity	0.06	0.00	0.01				
Queue Length 95th (ft)	0.00	0.00	1				
Control Delay (s)	0.0	0.3	9.2				
Lane LOS	0.0	Α	Α.2				
Approach Delay (s)	0.0	0.3	9.2				
Approach LOS	0.0	0.5	9.2 A				
			М				
Intersection Summary							
Average Delay			0.5				
Intersection Capacity Utilizat	tion		18.2%	IC	U Level o	of Service	
Analysis Period (min)			15				

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Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	1	I SAME		सी	N.	
Traffic Volume (veh/h)	84	2	1	97	2	1
Future Volume (Veh/h)	84	2	1	97	2	1
Sign Control	Free		(7)	Free	Stop	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	91	2	1	105	2	1
Pedestrians				,,,,		
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	None			None		
Median storage veh)	140116			140110		
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume			93		199	92
vC1, stage 1 conf vol			30		100	32
vC2, stage 2 conf vol						
vCu, unblocked vol			93		199	92
			4.1		6.4	6.2
tC, single (s)			4.1		0.4	0.2
tC, 2 stage (s)			2.2		2.5	3.3
tF (s)					3.5	
p0 queue free %			100		100	100
cM capacity (veh/h)			1514		794	971
Direction, Lane #	EB 1	WB 1	NB1	1100		fix Su 🖹
Volume Total	93	106	3			
Volume Left	0	1	2			
Volume Right	2	0	1			
cSH	1700	1514	845			
Volume to Capacity	0.05	0.00	0.00			
Queue Length 95th (ft)	0	0	0			
Control Delay (s)	0.0	0.1	9.3			
Lane LOS		Α	Α			
Approach Delay (s)	0.0	0.1	9.3			
Approach LOS			Α			
Intersection Summary	Merse A	STATE				
Average Delay			0.2			
Intersection Capacity Utiliz	zation		15.9%	IC	U Level o	f Service
Analysis Period (min)			15			
/ maryora r oriod (min)			10			

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Movement	WBL	WBR	NBT	NBR	SBL	SET	3.2
Lane Configurations	W		44	The same of the	7	^	
Traffic Volume (veh/h)	21	75	495	29	72	395	
Future Volume (Veh/h)	21	75	495	29	72	395	
Sign Control	Stop		Free			Free	
Grade	0%		0%			0%	
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Hourly flow rate (vph)	21	75	495	29	72	395	
Pedestrians	1						
Lane Width (ft)	12.0						
Walking Speed (ft/s)	4.0						
Percent Blockage	0						
Right turn flare (veh)							
Median type			None			None	
Median storage veh)							
Upstream signal (ft)							
pX, platoon unblocked							
vC, conflicting volume	852	263			525		
vC1, stage 1 conf vol							
vC2, stage 2 conf vol							
vCu, unblocked vol	852	263			525		
tC, single (s)	6.8	6.9			4.2		
tC, 2 stage (s)	0.0	0.0			1.6		
tF (s)	3.5	3.3			2.2		
p0 queue free %	93	90			93		
cM capacity (veh/h)	281	741			1023		
			AID O	004		25.0	
Direction, Lane #	WB 1	NB 1	NB 2	SB 1	SB 2	SB 3	
Volume Total	96	330	194	72	198	198	
Volume Left	21	0	0	72	0	0	
Volume Right	75	0	29	0	0	0	
cSH	546	1700	1700	1023	1700	1700	
Volume to Capacity	0.18	0.19	0.11	0.07	0.12	0.12	
Queue Length 95th (ft)	16	0	0	6	0	0	
Control Delay (s)	13.0	0.0	0.0	8.8	0.0	0.0	
Lane LOS	В			Α			
Approach Delay (s)	13.0	0.0		1.4			
Approach LOS	В						
Intersection Summary		1					
Average Delay			1.7				4
Intersection Capacity Utiliza	ition		34.4%	IC	U Level	of Service	е
Analysis Period (min)			15				



KITTELSON & ASSOCIATES, INC.

610 SW Alder, Suite 700 Portland, Oregon 97205 (503) 228-5230 Fax: (503) 273-8169

 Project #:
 19056

 Project Name:
 SOZO Sports

 Analyst:
 BHR

 Date:
 2/4/2016

File:

Intersection: Ahtanum Road/S 38th Ave

Scenario: Phase 1

Warrant Summary

Warrant	Name	Analyzed?	Met?
#1	Eight-Hour Vehicular Volume	Yes	No
#2	Four-Hour Vehicular volume	Yes	No
#3	Peak Hour	Yes	No
#4	Pedestrian Volume	No	*
#5	School Crossing	No	
#6	Coordinated Signal System	No	*
#7	Crash Experience	No	*
#8	Roadway Network	No	

Input Parameters Volume Adjustment Factor = 1.0 North-South Approach = minor East-West Approach = Major Major Street Thru Lanes = 1 Minor Street Thru Lanes = 1 Speed > 40 mph? Yes Population < 10,000? No Warrant Factor 70% Peak Hour or Dally Count? Daily

Analysis Traffic Volumes

ŀ	lour	Major	Street	Minor	or Street	
Begin	End	EB	WB	NB	SB	
12:00 AM	1:00 AM	10	25	0	2	
1:00 AM	2:00 AM	7	13	0	2	
2:00 AM	3:00 AM	11	25	0	2	
3:00 AM	4:00 AM	25	15	0	1	
4:00 AM	5:00 AM	76	115	0	10	
5:00 AM	6:00 AM	168	190	0	21	
6:00 AM	7:00 AM	377	166	0	36	
7:00 AM	8:00 AM	548	176	0	59	
8:00 AM	9:00 AM	359	193	0	44	
9:00 AM	10:00 AM	276	214	0	41	
10:00 AM	11:00 AM	247	253	0	47	
11:00 AM	12:00 PM	242	278	0	54	
12:00 PM	1:00 PM	249	289	0	49	
1:00 PM	2:00 PM	248	294	0	28	
2:00 PM	3:00 PM	317	381	0	40	
3:00 PM	4:00 PM	325	491	0	64	
4:00 PM	5:00 PM	335	542	0	62	
5:00 PM	6:00 PM	332	522	0	67	
6:00 PM	7:00 PM	195	350	0	31	
7:00 PM	8:00 PM	175	204	0	36	
8:00 PM	9:00 PM	119	195	0	18	
9:00 PM	10:00 PM	109	183	0	18	
10:00 PM	11:00 PM	72	85	0	12	
11:00 PM	12:00 AM	29	61	0	5	

			Warrant #1 - E	ight Hour		
Warrant Factor	Condition	Major Street Requirement	Minor Street Requirement	Hours That Condition Is Met	Condition for Warrant Factor Met?	Signal Warrant Met?
1000/	А	500	150	0	No	N-
100%	В	750	75	0	No	No
0001	A	400	120	0	No	
80%	В	600	60	3	No	No
700/	Α	350	105	0	No	N.
70%	В	525	53	4	No	No



Appendix B Phase 2 Operations Worksheets

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	٦	→	4	•	~	4	
Movement	EBL	EBT	WBT	WBR	SBL	SBR	
Lane Configurations		स	î»	1 September 2	A.		
Traffic Volume (veh/h)	40	353	542	131	173	27	
Future Volume (Veh/h)	40	353	542	131	173	27	
Sign Control		Free	Free		Stop		
Grade		0%	0%		0%		
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Hourly flow rate (vph)	40	353	542	131	173	27	
Pedestrians							
Lane Width (ft)							
Walking Speed (ft/s)							
Percent Blockage							
Right turn flare (veh)							
Median type		None	None				
Median storage veh)		None	NONE				
Upstream signal (ft)							
pX, platoon unblocked							
vC, conflicting volume	673				1040	608	
	0/3				1040	000	
vC1, stage 1 conf vol							
vC2, stage 2 conf vol	672				1040	600	
vCu, unblocked vol	673					608	
tC, single (s)	4.2				6.4	6.2	
tC, 2 stage (s)	0.0				0.5	0.0	
tF (s)	2.3				3.5	3.3	
p0 queue free %	96				29	95	
cM capacity (veh/h)	890				243	500	
Direction, Lane #	EB 1	V/B 1	SB 1		150		
Volume Total	393	673	200				
Volume Left	40	0	173				
Volume Right	0	131	27				
cSH	890	1700	262				
Volume to Capacity	0.04	0.40	0.76				
Queue Length 95th (ft)	4	0	141				
Control Delay (s)	1.4	0.0	52.6				
Lane LOS	Α		F				
Approach Delay (s)	1.4	0.0	52.6				
Approach LOS			F				
Intersection Summary	all Assets	4 %		WHI S		TIE T	
Average Delay			8.8				
Intersection Capacity Utilizati	ion		69.9%	ıc	U Level o	f Service	
	1011			IC	O LEVEI U	OCI VICE	14
Analysis Period (min)			15				

	-	*	•	•	4	-		
Movement	EBT	EBR	WBL	WBT	NBL	NBR		
Lane Configurations	1>			र्स	NA.	Self South		
Traffic Volume (veh/h)	0	47	133	0	95	38		
Future Volume (Veh/h)	0	47	133	0	95	38		
Sign Control	Free			Free	Yield			
Grade	0%			0%	0%			
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00		
Hourly flow rate (vph)	0	47	133	0	95	38		
Pedestrians			100		-	00		
Lane Width (ft)								
Walking Speed (ft/s)								
Percent Blockage								
Right turn flare (veh)								
Median type	None			None				
Median storage veh)	140116			HOHE				
Upstream signal (ft)								
pX, platoon unblocked								
vC, conflicting volume			47		290	24		
vC1, stage 1 conf vol			47		290	24		
vC2, stage 2 conf vol			47		200	0.4		
vCu, unblocked vol			47		290	24		
tC, single (s)			4.1		6.4	6.2		
tC, 2 stage (s)			0.0					
tF (s)			2.2		3.5	3.3		
p0 queue free %			91		85	96		
cM capacity (veh/h)			1560		641	1053		
Direction, Lane #	EB 1	WB 1	NB 1					
Volume Total	47	133	133					
Volume Left	0	133	95					
Volume Right	47	0	38					
cSH	1700	1560	722					
Volume to Capacity	0.03	0.09	0.18					
Queue Length 95th (ft)	0	7	17					
Control Delay (s)	0.0	7.5	11.1					
Lane LOS		Α	В					
Approach Delay (s)	0.0	7.5	11.1					
Approach LOS			В					
Intersection Summary		1						
Average Delay			7.9					
Intersection Capacity Utiliza	tion		28.3%	IC	U Level o	of Service	- A	
Analysis Period (min)			15					

	۶	-	•	•	-	4		
Movement	EBL	EBT	WBT	WBR	SBL	SBR		i Ville
Lane Configurations		र्स	ĵ.		A	The state of the s		
Traffic Volume (veh/h)	41	418	276	112	121	34		
Future Volume (Veh/h)	41	418	276	112	121	34		
Sign Control		Free	Free		Stop			
Grade		0%	0%		0%			
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00		
lourly flow rate (vph)	41	418	276	112	121	34		
edestrians								
ane Width (ft)								
/alking Speed (ft/s)								
ercent Blockage								
ight turn flare (veh)								
edian type		None	None					
ledian storage veh)								
pstream signal (ft)								
X, platoon unblocked								
C, conflicting volume	388				832	332		
C1, stage 1 conf vol								
22, stage 2 conf vol								
Cu, unblocked vol	388				832	332		
, single (s)	4.2				6.4	6.2		
, 2 stage (s)					• • • • • • • • • • • • • • • • • • • •			
(s)	2.3				3.5	3.3		
) queue free %	96				63	95		
/ capacity (veh/h)	1138				327	714		
rection, Lane #	EB 1	WB 1	SB 1	= 70 =	V S S S		2 - 2 W 1 S	
ume Total	459	388	155					
olume Left	41	0	121					
olume Right	0	112	34					
SH	1138	1700	371					
olume to Capacity	0.04	0.23	0.42					
Jeue Length 95th (ft)	3	0.23	50					
ontrol Delay (s)	1.1	0.0	21.5					
ine LOS	Α	0.0	21.5 C					
proach Delay (s)	1.1	0.0	21.5					
pproach LOS	1.1	0.0	Z1.5					
			U					
tersection Summary		14 15 15		W 8 1	1.40	L = 1/2		
verage Delay			3.8					
tersection Capacity Utilizati	ion		64.4%	IC	U Level o	f Service		С
nalysis Period (min)			15					

	-	•	•	←	4	-
Movement	EBT	EBR	Wal	WBT	NBL	NBR
Lane Configurations	ĵ.			4	Y	U.S. Allo
Traffic Volume (veh/h)	0	126	6	0	117	10
Future Volume (Veh/h)	0	126	6	0	117	10
Sign Control	Free			Free	Yield	
Grade	0%			0%	0%	
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Hourly flow rate (vph)	0	126	6	0	117	10
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	None			None		
Median storage veh)	110110			110110		
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume			126		75	63
vC1, stage 1 conf vol			120		73	03
vC1, stage 1 conf vol						
vCu, unblocked vol			126		75	63
			4.1		6.4	6.2
tC, single (s) tC, 2 stage (s)			4.1		0.4	0.2
			2.2		2.5	2.2
tF (s) p0 queue free %			100		3.5	3.3 99
			1460		87 925	1002
cM capacity (veh/h)					925	1002
Direction, Lane #	EB 1	WB 1	NB 1			
Volume Total	126	6	127			
Volume Left	0	6	117			
Volume Right	126	0	10			
cSH	1700	1460	930			
Volume to Capacity	0.07	0.00	0.14			
Queue Length 95th (ft)	0	0	12			
Control Delay (s)	0.0	7.5	9.5			
Lane LOS		Α	Α			
Approach Delay (s)	0.0	7.5	9.5			
Approach LOS			Α			
Intersection Summary						
Average Delay			4.8			
Intersection Capacity Utilization	ation		21.6%	IC	III evel d	of Service
Analysis Period (min)			15		O LOVER C	, CCI VICE
Alialysis Fellou (Illiff)			13			

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KITTELSON & ASSOCIATES, INC.

610 SW Alder, Suite 700 Portland, Oregon 97205 (503) 228-5230 Fax: (503) 273-8169

Project #: Project Name: 19056

Analyst:

SOZO Sports BHR

Date:

2/4/2016

File:

* Tot, Perford profiled LBOG- to the Spectra Completed and Special Workshop LBOG Special Workshop LBOG Special Speci

Intersection:

Ahtanum Road/S 38th Ave

Scenario:

Phase 2

Warrant	Summary
---------	----------------

Warrant	Name	Analyzed?	Met?
#1	Eight-Hour Vehicular Volume	Yes	Yes
#2	Four-Hour Vehicular volume	Yes	Yes
#3	Peak Hour	Yes	Yes*
#4	Pedestrian Volume	No	97
#5	School Crossing	No	.51
#6	Coordinated Signal System	No	327
#7	Crash Experience	No	190
#8	Roadway Network	No	(3)

Input	Para	neters
-------	------	--------

Volume Adjustment Factor =	1.0
North-South Approach =	minor
East-West Approach =	Major
Major Street Thru Lanes =	1
Minor Street Thru Lanes =	1
Speed > 40 mph?	Yes
Population < 10,000?	No
Warrant Factor	70%
Peak Hour or Daily Count?	Daily

Analysis Traffic Volumes

	Hour	Major Street		Minor Street		
Begin	End	EB	WB	NB	SB	
12:00 AM	1:00 AM	11	26	0	6	
1:00 AM	2:00 AM	8	14	0	4	
2:00 AM	3:00 AM	12	26	0	6	
3:00 AM	4:00 AM	26	16	O	5	
4:00 AM	5:00 AM	82	122	O	30	
5:00 AM	6:00 AM	179	203	0	59	
6:00 AM	7:00 AM	394	185	0	93	
7:00 AM	8:00 AM	571	202	0	136	
8:00 AM	9:00 AM	376	213	0	102	
9:00 AM	10:00 AM	291	232	0	93	
10:00 AM	11:00 AM	263	271	0	101	
11:00 AM	12:00 PM	259	297	0	110	
12:00 PM	1:00 PM	266	308	0	107	
1:00 PM	2:00 PM	264	313	0	84	
2:00 PM	3:00 PM	338	405	0	112	
3:00 PM	4:00 PM	350	520	0	150	
4:00 PM	5:00 PM	362	573	0	154	
5:00 PM	6:00 PM	358	553	0	157	
6:00 PM	7:00 PM	212	369	0	88	
7:00 PM	8:00 PM	187	218	0	77	
8:00 PM	9:00 PM	128	206	0	51	
9:00 PM	10:00 PM	118	193	0	48	
10:00 PM	11:00 PM	77	90	0	29	
11:00 PM	12:00 AM	31	65	0	15	

Warrant	#1	Cink	P Marie

Warrant Factor	Condition	Major Street Requirement	Minor Street Requirement	Hours That Condition Is Met	Condition for Warrant Factor Met?	Signal Warrant Met?
100%	Α	500	150	3	No	,u N-
100%	В	750	75	4	No	No
D004	A	400	120	4	No	
80%	В	600	60	5	No	No
70%	Α	350	105	7	No	
70%	В	525	53	12	Yes	Yes

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Appendix C Phase 3 Operations Worksheets

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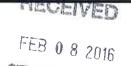
	۶	-	←	4	~	4
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		4	1	NO-S/BN	W	A. C.
Traffic Volume (veh/h)	19	353	542	158	115	26
Future Volume (Veh/h)	19	353	542	158	115	26
Sign Control		Free	Free		Stop	
Grade		0%	0%		0%	
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Hourly flow rate (vph)	19	353	542	158	115	26
Pedestrians			0.2			
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type		None	None			
Median storage veh)		None	None			
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	700				1012	621
	700				1012	021
vC1, stage 1 conf vol vC2, stage 2 conf vol						
vCu, unblocked vol	700				1012	621
	4.2				6.4	6.2
tC, single (s)	4.2				0.4	0.2
tC, 2 stage (s)	0.0				2.5	2.2
tF (s)	2.3				3.5	3.3
p0 queue free %	98				56	95
cM capacity (veh/h)	870				259	491
Direction, Lane#	EB 1	WB 1	SB 1			
Volume Total	372	700	141			
Volume Left	19	0	115			
Volume Right	0	158	26			
cSH	870	1700	284			
Volume to Capacity	0.02	0.41	0.50			
Queue Length 95th (ft)	2	0	65			
Control Delay (s)	0.7	0.0	29.5			
Lane LOS	A		D			
Approach Delay (s)	0.7	0.0	29.5			
Approach LOS			D			
Intersection Summary	a li w		U	13	-	- 1
Average Delay			3.7			
Intersection Capacity Utiliza	ation		52.8%	IC	U Level o	f Service
Analysis Period (min)	alion		15	- 10	O LCVCI C	A OCI VIOC
Analysis Feriou (min)			10			

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	-	*	•	-	4	-	
Movement	EBT	EBR	WBL	WBT	NBL	NBR	
Lane Configurations	1		The same of the sa	र्स	M		
Traffic Volume (veh/h)	0	0	120	0	0	139	
Future Volume (Veh/h)	0	0	120	0	0	139	
Sign Control	Free			Free	Yield		
Grade	0%			0%	0%		
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Hourly flow rate (vph)	0	0	120	0	0	139	
Pedestrians							
Lane Width (ft)							
Walking Speed (ft/s)							
Percent Blockage							
Right turn flare (veh)							
Median type	None			None			
Median storage veh)							
Upstream signal (ft)							
pX, platoon unblocked							
vC, conflicting volume			0		240	0	
vC1, stage 1 conf vol						The state of	
vC2, stage 2 conf vol							
vCu, unblocked vol			0		240	0	
tC, single (s)			4.1		6.4	6.2	
tC, 2 stage (s)							
tF (s)			2.2		3.5	3.3	
p0 queue free %			93		100	87	
cM capacity (veh/h)			1623		693	1085	
Direction, Lane #	EB 1	WB 1	NB 1			100.0	
Volume Total	0	120	139				
Volume Left	0	120	0				
Volume Right	0	0	139				
cSH	1700	1623	1085				
Volume to Capacity	0.00	0.07	0.13				
Queue Length 95th (ft)	0.00	6	11				
Control Delay (s)	0.0	7.4	8.8				
Lane LOS	0.0	A	Α.				
Approach Delay (s)	0.0	7.4	8.8				
Approach LOS	0.0	1.7	Α				
Intersection Summary			2				
Average Delay			8.2				
Intersection Capacity Utilizat	tion		21.9%	IC	U Level o	of Service	
Analysis Period (min)	11VII		15	10	O LUVEI (OCI VICE	
Alialysis Fellou (IIIIII)			10				

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rvice	ce
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	-	*	•	•	4	~	
Movement	EBT	EBR	WBL	WBT	NBL	NBR	
Lane Configurations	1		7	^	W	110. 110.	
Traffic Volume (veh/h)	164	7	7	146	5	4	
Future Volume (Veh/h)	164	7	7	146	5	4	
Sign Control	Free			Free	Stop		
Grade	0%			0%	0%		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	
Hourly flow rate (vph)	178	8	8	159	5	4	
Pedestrians							
Lane Width (ft)							
Walking Speed (ft/s)							
Percent Blockage							
Right turn flare (veh)							
Median type	TWLTL			TWLTL			
Median storage veh)	2			2			
Upstream signal (ft)	_			_			
pX, platoon unblocked							
vC, conflicting volume			186		357	182	
vC1, stage 1 conf vol					182		
vC2, stage 2 conf vol					175		
vCu, unblocked vol			186		357	182	
tC, single (s)			4.1		6.4	6.2	
tC, 2 stage (s)					5.4		
tF (s)			2.2		3.5	3.3	
p0 queue free %			99		99	100	
cM capacity (veh/h)			1401		765	866	
Direction, Lane #	EB 1	WB 1	WB 2	NB 1			
Volume Total	186	8	159	9			
Volume Left	0	8	0	5			
Volume Right	8	0	0	4			
cSH	1700	1401	1700	807			
Volume to Capacity	0.11	0.01	0.09	0.01			
Queue Length 95th (ft)	0.11	0.01	0.03	1			
Control Delay (s)	0.0	7.6	0.0	9.5			
Lane LOS	0.0	Α.	0.0	Α.			
Approach Delay (s)	0.0	0.4		9.5			
Approach LOS	0.0	0.4		9.5 A			
Intersection Summary							
Average Delay			0.4				
Intersection Capacity Utiliz	ation		19.1%	IC	U Level o	of Service	
Analysis Period (min)			15				



	-	*	•	-	4	-	
Movement	EBT	EBR	WBL	WBT	NBL	NBR	
Lane Configurations	f	255 1044 105	7	^	N/		
Traffic Volume (veh/h)	166	2	1	152	1	0	
Future Volume (Veh/h)	166	2	1	152	1	0	
Sign Control	Free			Free	Stop		
Grade	0%			0%	0%		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	
Hourly flow rate (vph)	180	2	1	165	1	0	
Pedestrians							
Lane Width (ft)							
Walking Speed (ft/s)							
Percent Blockage							
Right turn flare (veh)							
Median type	TWLTL			TWLTL			
Median storage veh)	2			2			
Upstream signal (ft)							
pX, platoon unblocked							
vC, conflicting volume			182		348	181	
vC1, stage 1 conf vol					181		
vC2, stage 2 conf vol					167		
vCu, unblocked vol			182		348	181	
tC, single (s)			4.1		6.4	6.2	
tC, 2 stage (s)					5.4		
tF (s)			2.2		3.5	3.3	
p0 queue free %			100		100	100	
cM capacity (veh/h)			1405		773	867	
Direction, Lane #	EB 1	WB 1	WB 2	NB 1	4 7 31		Company of the State of the Company
Volume Total	182	1	165	1			
Volume Left	0	i	0	1			
Volume Right	2	Ö	0	Ö			
cSH	1700	1405	1700	773			
Volume to Capacity	0.11	0.00	0.10	0.00			
Queue Length 95th (ft)	0.11	0.00	0.10	0.00			
Control Delay (s)	0.0	7.6	0.0	9.7			
Lane LOS	0,0	Α.	3.0	Α.			
Approach Delay (s)	0.0	0.0		9.7			
Approach LOS	0.0	3.0		A			
Intersection Summary		(bygge		10 X - 10 X		W 12.00	
Average Delay			0.0			- VV	
Intersection Capacity Utiliz	ation		18.9%	IC	U Level o	f Service	Α
Analysis Period (min)			15				

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Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	1>		ሻ	↑	14	7,100,1
Traffic Volume (veh/h)	158	8	8	148	5	4
Future Volume (Veh/h)	158	8	8	148	5	4
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	172	9	9	161	5	4
Pedestrians	,,,_					
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	TWLTL			TWLTL		
Median storage veh)	2			2		
Upstream signal (ft)						
pX, platoon unblocked			404		250	470
vC, conflicting volume			181		356	176
vC1, stage 1 conf vol					176	
vC2, stage 2 conf vol					179	
vCu, unblocked vol			181		356	176
tC, single (s)			4.1		6.4	6.2
tC, 2 stage (s)					5.4	
tF (s)			2.2		3.5	3.3
p0 queue free %			99		99	100
cM capacity (veh/h)			1407		766	872
Direction, Lane #	EB 1	WB1	WB 2	NB 1	- 6	
Volume Total	181	9	161	9		
Volume Left	0	9	0	5		
Volume Right	9	0	0	4		
cSH	1700	1407	1700	809		
Volume to Capacity	0.11	0.01	0.09	0.01		
Queue Length 95th (ft)	0	0	0	1		
Control Delay (s)	0.0	7.6	0.0	9.5		
Lane LOS		Α		A		
Approach Delay (s)	0.0	0.4		9.5		
Approach LOS	- 0.0	0.1		A		
Intersection Summary				-		100
Average Delay			0.4			
Intersection Capacity Utiliz	zation		18.8%	IC	U Level o	of Consinc
	Latiuri			IU	O Level 0	o Service
Analysis Period (min)			15			

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Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	1>		*	^	W	
Traffic Volume (veh/h)	157	5	5	150	6	5
Future Volume (Veh/h)	157	5	5	150	6	5
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	171	5	5	163	7	5
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	TWLTL			TWLTL		
Median storage veh)	2			2		
Upstream signal (ft)	_			_		
pX, platoon unblocked						
vC, conflicting volume			176		346	174
vC1, stage 1 conf vol					174	
vC2, stage 2 conf vol					173	
vCu, unblocked vol			176		346	174
tC, single (s)			4.1		6.4	6.2
tC, 2 stage (s)			7.1		5.4	U.L
tF (s)			2.2		3.5	3.3
p0 queue free %			100		99	99
cM capacity (veh/h)			1412		772	875
	THE PERSON NAMED IN	AA III A		1000	112	0/0
Direction, Lane #	EB 1	WB 1	WB 2	NB 1	E WIN	
Volume Total	176	5	163	12		
Volume Left	0	5	0	7		
Volume Right	5	0	0	5		
cSH	1700	1412	1700	812		
Volume to Capacity	0.10	0.00	0.10	0.01		
Queue Length 95th (ft)	0	0	0	1		
Control Delay (s)	0.0	7.6	0.0	9.5		
Lane LOS		Α		Α		
Approach Delay (s)	0.0	0.2		9.5		
Approach LOS				Α		
Intersection Summary					1 4	8 F, F
Average Delay			0.4			
Intersection Capacity Utiliz	zation		18.6%	IC	U Level o	f Service
Analysis Period (min)			15			

	-	•	•	-		-	
Movement	EBT	EBR	WBL	WBT	NBL	NBR	Ħ
Lane Configurations	f)	0.000(0.0)	7	↑	W	Non-Aven	_
Traffic Volume (veh/h)	160	2	2	154	- 1	1	
Future Volume (Veh/h)	160	2	2	154	1	1	
Sign Control	Free			Free	Stop		
Grade	0%			0%	0%		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	
Hourly flow rate (vph)	174	2	2	167	1	1	
Pedestrians			<u>-</u>				
Lane Width (ft)							
Walking Speed (ft/s)							
Percent Blockage							
Right turn flare (veh)							
Median type	TWLTL			TWLTL			
Median storage veh)	2			2			
Upstream signal (ft)	_						
pX, platoon unblocked							
vC, conflicting volume			176		346	175	
vC1, stage 1 conf vol			- 113		175	110	
vC2, stage 2 conf vol					171		
vCu, unblocked vol			176		346	175	
tC, single (s)			4.1		6.4	6.2	
tC, 2 stage (s)					5.4	0.2	
tF(s)			2.2		3.5	3.3	
p0 queue free %			100		100	100	
cM capacity (veh/h)			1412		773	874	
	-	NAME OF		Name of the last	710	0/4	
Direction, Lane #	EB 1	WB 1	WB 2	NB 1			
Volume Total	176	2	167	2			
Volume Left	0	2	0	1_			
Volume Right	2	0	0	1			
cSH	1700	1412	1700	820			
Volume to Capacity	0.10	0.00	0.10	0.00			
Queue Length 95th (ft)	0	0	0	0			
Control Delay (s)	0.0	7.6	0.0	9.4			
Lane LOS		Α		Α			
Approach Delay (s)	0.0	0.1		9.4			
Approach LOS				Α			
Intersection Summary					778		
Average Delay			0.1				
Intersection Capacity Utiliza	ation		18.5%	IC	U Level o	f Service	
Analysis Period (min)			15				

	۶	-	-	•	-	1		
Movement	EBL	EBT	WBT	WBR	SBL	SBR	MALES IN	
Lane Configurations		र्स	1		M			
Traffic Volume (veh/h)	26	418	276	144	151	27		
Future Volume (Veh/h)	26	418	276	144	151	27		
Sign Control		Free	Free		Stop			
Grade		0%	0%		0%			
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00		
Hourly flow rate (vph)	26	418	276	144	151	27		
Pedestrians								
Lane Width (ft)								
Walking Speed (ft/s)								
Percent Blockage								
Right turn flare (veh)								
Median type		None	None					
Median storage veh)								
Upstream signal (ft)								
pX, platoon unblocked								
vC, conflicting volume	420				818	348		
vC1, stage 1 conf vol								
vC2, stage 2 conf vol								
vCu, unblocked vol	420				818	348		
tC, single (s)	4.2				6.4	6.2		
tC, 2 stage (s)								
tF (s)	2.3				3.5	3.3		
p0 queue free %	98				55	96		
cM capacity (veh/h)	1108				337	700		
Direction, Lane #	EB 1	WB 1	SB 1	-V	WILL.	5 15		
Volume Total	444	420	178					
Volume Left	26	0	151					
Volume Right	0	144	27					
cSH	1108	1700	366					
Volume to Capacity	0.02	0.25	0.49					
Queue Length 95th (ft)	2	0.20	64					
Control Delay (s)	0.7	0.0	23.8					
Lane LOS	A	0.0	C					
Approach Delay (s)	0.7	0.0	23.8					
Approach LOS	0.1	0.0	C					
Intersection Summary	D-1 1	rą icz	THE R		No.	ورقال کا		
Average Delay			4.4					
Intersection Capacity Utiliza	ntion		60.0%	IC	U Level o	f Service		
Analysis Period (min)	NOTE OF THE PERSON OF THE PERS		15	10	O LOVOI O	, JOI VICE		
anaiyoio Feniuu (IIIIII)			10					

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Movement	EBT	EBR	WBL	WBT	NBL	NBR	
Lane Configurations	1>	The Paris III.	TIMESES.	र्भ	W	STORY AND	
Traffic Volume (veh/h)	0	0	212	0	0	177	
Future Volume (Veh/h)	0	0	212	0	0	177	
Sign Control	Free			Free	Yield		
Grade	0%			0%	0%		
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Hourly flow rate (vph)	0	0	212	0	0	177	
Pedestrians							
Lane Width (ft)							
Walking Speed (ft/s)							
Percent Blockage							
Right turn flare (veh)							
Median type	None			None			
Median storage veh)							
Upstream signal (ft)							
pX, platoon unblocked							
vC, conflicting volume			0		424	0	
vC1, stage 1 conf vol							
vC2, stage 2 conf vol							
vCu, unblocked vol			0		424	0	
tC, single (s)			4.1		6.4	6.2	
tC, 2 stage (s)							
tF (s)			2.2		3.5	3.3	
p0 queue free %			87		100	84	
cM capacity (veh/h)			1623		510	1085	
Direction, Lane #	EB 1	WB 1	NB 1	-17			
Volume Total	0	212	177				
Volume Left	0	212	0				
Volume Right	0	0	177				
cSH	1700	1623	1085				
Volume to Capacity	0.00	0.13	0.16				
Queue Length 95th (ft)	0	11_	15				
Control Delay (s)	0.0	7.6	9.0				
Lane LOS		Α	Α				
Approach Delay (s)	0.0	7.6	9.0				
Approach LOS			Α				
Intersection Summary					st file	بارد المتحد	
Average Delay			8.2				
Intersection Capacity Utilizati	ion		29.4%	IC	U Level o	f Service	
Analysis Period (min)			15				

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Movement	EBL	EBR	NBL	NBT	SBT	SBR	10
Lane Configurations	7	7	11000	र्स	1>		
Traffic Volume (veh/h)	150	150	138	32	28	138	
Future Volume (Veh/h)	150	150	138	32	28	138	
Sign Control	Stop			Free	Free		
Grade	0%			0%	0%		
Peak Hour Factor	0.80	0.80	0.80	0.80	0.80	0.80	
Hourly flow rate (vph)	188	188	173	40	35	173	
Pedestrians	100	100	110		00		
Lane Width (ft)							
Walking Speed (ft/s)							
Percent Blockage							
Right turn flare (veh)							
Median type				None	None		
Median storage veh)				None	None		
Upstream signal (ft)							
pX, platoon unblocked							
vC, conflicting volume	508	122	208				
vC1, stage 1 conf vol	300	122	200				
vC1, stage 1 conf vol							
vCu, unblocked vol	508	122	208				
	6.4	6.2	4.1				
tC, single (s)	0.4	0.2	4.1				
tC, 2 stage (s)	2.5	2.2	0.0				
tF (s)	3.5	3.3	2.2				
p0 queue free %	59	80	87				
cM capacity (veh/h)	462	935	1375				
Direction, Lane #	EB 1	EB 2	NB 1	SB 1			
Volume Total	188	188	213	208			
Volume Left	188	0	173	0			
Volume Right	0	188	0	173			
cSH	462	935	1375	1700			
Volume to Capacity	0.41	0.20	0.13	0.12			
Queue Length 95th (ft)	49	19	11	0			
Control Delay (s)	18.0	9.8	6.7	0.0			
Lane LOS	С	Α	Α				
Approach Delay (s)	13.9		6.7	0.0			
Approach LOS	В						
Intersection Summary				TAKE	SIN Y	Jan N.	P
Average Delay			8.4				
Intersection Capacity Utilization	ation		37.6%	IC	U Level o	f Service	
Analysis Period (min)	G11011		15		J LOVOI U	, JOI VIOC	
Alialysis Fellou (Illill)			10				

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Movement	EBT	EBR	WBL	WBT	NBL	NBR	90
Lane Configurations	1		4	^	W		
Traffic Volume (veh/h)	154	6	6	178	5	5	
Future Volume (Veh/h)	154	6	6	178	5	5	
Sign Control	Free			Free	Stop		
Grade	0%			0%	0%		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	
Hourly flow rate (vph)	167	7	7	193	5	5	
Pedestrians							
Lane Width (ft)							
Walking Speed (ft/s)							
Percent Blockage							
Right turn flare (veh)							
Median type	TWLTL			TWLTL			
Median storage veh)	2			2			
Upstream signal (ft)	_			_			
pX, platoon unblocked							
vC, conflicting volume			174		378	170	
vC1, stage 1 conf vol					170		
vC2, stage 2 conf vol					207		
vCu, unblocked vol			174		378	170	
tC, single (s)			4.1		6.4	6.2	
tC, 2 stage (s)					5.4	V.L	
tF (s)			2.2		3.5	3.3	
p0 queue free %			100		99	99	
cM capacity (veh/h)			1415		753	879	
	-	4181800 A		181998779	700	0/0	
Direction, Lane #	EB 1	WB 1	WB 2	NB 1			
Volume Total	174	7	193	10			
Volume Left	0	7	0	5			
Volume Right	7	0	0	5			
cSH	1700	1415	1700	811			
Volume to Capacity	0.10	0.00	0.11	0.01			
Queue Length 95th (ft)	0	0	0	1			
Control Delay (s)	0.0	7.6	0.0	9.5			
Lane LOS		Α		Α			
Approach Delay (s)	0.0	0.3		9.5			
Approach LOS				Α			
Intersection Summary			1,4				
Average Delay			0.4				
Intersection Capacity Utiliz	zation		19.4%	IC	U Level o	f Service	
Analysis Period (min)			15		2 20.010	. 5011100	
, mary sis i crioù (mill)			13				

Movement		-	•	•	4	4	-	
Lane Configurations Traffic Volume (veh/h) 158 1 1 183 1 1 Sign Control Free Grade 0% 0% 0% 0% 0% Peak Hour Factor 0.92 0.93 0.93 0.94 1 1 1 1 1 1 1 1 1 1 1 1 1	Movement	EBT	EBR	WBL	WBT	NBL	NBR	
Traffic Volume (veh/h)		1>		ħ	1	A		
Future Volume (Veh/h) 158 1 1 183 1 1 1 Sign Control Free Free Stop Grade 0% 0% 0% 0% 0% 0% 0% 0% 0% 0% 0% 0% 0%		158	1	1	183	1	1	
Free Grade O% O% O% O% O% O% O% O				1		1		
Grade 0% 0% 0% 0% Peak Hour Factor 0.92 0.92 0.92 0.92 0.92 0.92 0.92 Peak Hour Factor 0.92 0.92 0.92 0.92 0.92 0.92 O.92 O.92 O.92 O.92 O.92 O.92 O.92 O					Free	Stop		
Peak Hour Factor 0.92 0.92 0.92 0.92 0.92 0.92 Hourly flow rate (vph) 172 1 1 199 1 1 Pedestrians Lane Width (ft) Walking Speed (ft/s) Percent Blockage Right turn flare (veh) Median storage veh) 2 2 Upstream signal (ft) pX, platoon unblocked vCc, conflicting volume 173 374 172 vC1, stage 1 conf vol 201 vC2, stage 2 conf vol 201 vC3, stage 2 conf vol 201 vC4, unblocked vol 173 374 172 tC5, single (s) 4.1 6.4 6.2 tC6, 2 stage (s) 5.4 tF (s) 2.2 3.5 3.3 p0 queue free % 100 100 100 cM capacity (veh/h) 1416 758 876 Direction, Lane # EB 1 WB 1 WB 2 NB 1 Volume Total 173 1 199 2 Volume Total 173 1 199 2 Volume Right 1 0 0 1 1 cSH 1700 1416 1700 813 Volume Right 1 0 0 0 1 cSH 1700 1416 1700 813 Volume Right 1 0 0 0 0 Control Detay (s) 0.0 7.5 0.0 9.4 Lane LOS A A A Approach Delay (s) 0.0 0.0 9.4 Approach LOS A Intersection Summary Average Delay Intersection Capacity Utilization 19.6% ICU Level of Service					0%			
Hourly flow rate (vph) 172 1 1 199 1 1 Pedestrians Lane Width (ft) Walking Speed (ft/s) Percent Blockage Right turn flare (veh) Median type TWLTL TWLTL Median storage veh) 2 2 Upstream signal (ft) pX, platoon unblocked vC, conflicting volume 173 374 172 vC1, stage 1 conf vol 172 vC2, stage 2 conf vol 201 vCu, unblocked vol 173 374 172 tC, single (s) 4.1 6.4 6.2 tC, 2 stage (s) 5.4 tF (s) 2.2 3.5 3.3 p0 queue free % 100 100 100 coft capacity (veh/h) 1416 758 876 Direction, Lane # EB 1 WB 1 WB 2 NB 1 Volume Total 173 1 199 2 Volume Right 1 0 0 1 coft Capacity (veh/h) 1416 1700 813 Volume Right 1 0 0 0 1 coft Capacity 0 0.10 0.00 0.12 Control Delay (s) 0.0 7.5 0.0 9.4 Lane LOS A A AApproach LoS A A Approach LoS A A Approach LoS A A Approach LoS A A Alteresection Summary Average Delay Intersection Summary Average Delay Intersection Capacity Utilization I 19.6% ICU Level of Service			0.92	0.92			0.92	
Pedestrians Lane Width (ft) Walking Speed (ft/s) Percent Blockage Right turn flare (veh) Median type TWLTL TWLTL Median storage veh) 2 2 Upstream signal (ft) pX, platoon unblocked vC, conflicting volume 173 374 172 vC1, stage 1 conf vol 172 vC2, stage 2 conf vol 201 vC2, stage 2 conf vol 201 vC3, single (s) 4.1 6.4 6.2 tC, c) single (s) 4.1 6.4 6.2 tC, 2 stage (s) 5.4 tF (s) 2.2 3.5 3.3 p0 queue free % 100 100 100 cont capacity (veh/h) 1416 758 876 Direction, Lane # EB 1 WB 1 WB 2 NB 1 Volume Total 173 1 199 2 Volume Left 0 1 0 1 volume Right 1 0 0 1 volume Right 1 0 0 1 volume Right 1 0 0 0 1 control Delay (s) 0.0 7.5 0.0 9.4 Approach Delay (s) 0.0 0.0 9.4 Approach LOS A A A Approach Delay (s) 0.0 0.0 9.4 Approach LOS A Intersection Summary Average Delay (utilization 19.6% ICU Level of Service 4.4)								
Lane Width (ft) Walking Speed (ft/s) Percent Blockage Right turn flare (veh) Median type TWLTL TWLTL Median storage veh) 2 2 Upstream signal (ft) XX, platoon unblocked XC1, stage 1 conf vol 172 XC2, stage 2 conf vol 201 XC4, unblocked vol 173 374 172 XC5, stage 2 conf vol 201 XC6, stage (s) 5.4 If (s) 2.2 3.5 3.3 XD0 queue free % 100 100 100 XDM capacity (veh/h) 1416 758 876 Direction, Lane # EB 1 WB 1 WB 2 NB 1 Volume Total 173 1 199 2 Volume Right 1 0 0 1 XD1 Volume Right 1 0 0 1 XD1 Volume Right 1 0 0 1 XD1 Volume Right 1 0 0 0 1 XD2 Queue Length 95th (ft) 0 0 0 0 Control Delay (s) 0.0 7.5 0.0 9.4 Approach LOS A A APproach Dolay (s) 0.0 0.0 9.4 Approach LOS A ICU Level of Service A IVULevel of Service			- "	1161			7.)	
Walking Speed (ft/s) Percent Blockage Right turn flare (veh) Median storage veh) 2 2 Upstream signal (ft) pX, platoon unblocked vC, conflicting volume 173 374 172 vC1, stage 1 conf vol vC2, stage 2 conf vol vC2, stage 2 conf vol tC, single (s) 4.1 6.4 6.2 tC, conflicting volume 173 374 172 tC, single (s) 4.1 6.4 6.2 tC, stage (s) 5.4 tF (s) 2.2 3.5 3.3 p0 queue free % 100 100 100 cM capacity (veh/h) 1416 758 876 Direction, Lane # EB 1 WB 1 WB 2 NB 1 Volume Total 173 1 199 2 Volume Left 0 1 0 1 Volume Right 1 0 0 1 cSH 1700 1416 1700 813 Volume to Capacity 0.10 0.00 0.12 0.00 Queue Length 95th (ft) 0 0 0 0 Control Delay (s) 0.0 7.5 0.0 9.4 Lane LOS A Approach Delay (s) 0.0 0.0 9.4 Approach LOS A Intersection Summary Average Delay Intersection Capacity Utilization 19.6% ICU Level of Service								
Percent Blockage Right tum flare (veh) Median type TWLTL TWLTL Median storage veh) 2 2 Upstream signal (ft) pX, platoon unblocked vC, conflicting volume 173 374 172 vC2, stage 1 conf vol 172 vC2, stage 2 conf vol 201 vC3, stage 1 conf vol 173 374 172 vC4, unblocked vol 173 374 172 vC6, unblocked vol 173 374 172 vC9, single (s) 4.1 6.4 6.2 vC9, stage (s) 5.4 vC9, stage (s) 6.2 vC9, stage (s)								
Right tum flare (veh) Median type TWLTL TWLTL Median storage veh) 2 2 Upstream signal (ft) px, platoon unblocked vC, conflicting volume 173 374 172 vC2, stage 1 conf vol vC2, stage 2 conf vol vC4, unblocked vol C5, stage 8 173 374 172 172 172 173 174 175 175 176 177 177 177 178 179 179 179 179								
Median type TWLTL TWLTL Median storage veh) 2 2 Upstream signal (ft) box, platoon unblocked vC, conflicting volume 173 374 172 vC1, stage 1 conf vol 201 172 202 201 vC2, stage 2 conf vol 201 201 202 202 202 202 202 202 202 202 202 203								
Median storage veh) 2 2 Upstream signal (ft) box, platoon unblocked VC, conflicting volume 173 374 172 VC1, stage 1 conf vol 201 172 VC2, stage 2 conf vol 201 201 VC2, stage (s) 4.1 6.4 6.2 C, 2 stage (s) 5.4 6.2 6.2 C, 2 stage (s) 5.4 6.2 6.2 F (s) 2.2 3.5 3.3 50 queue free % 100 100 100 5M capacity (veh/h) 1416 758 876 Direction, Lane # EB 1 WB 1 WB 2 NB 1 Volume Total 173 1 199 2 Volume Right 1 0 0 1 Volume Right 1 0 0 1 2SH 1700 1416 1700 813 Volume to Capacity 0.10 0.00 0 0 Queue Length 95th (ft) 0 0 0 0 Control Delay (s) 0.0		TWI TI			TWI TI			
Distream signal (ft) DX, platoon unblocked DX, p								
DX, platoon unblocked JC, conflicting volume 173 374 172 JC2, stage 1 conf vol 201 JC2, stage 2 conf vol 201 C, single (s) 4.1 6.4 6.2 C, single (s) 5.4 F (s) 5.4 F (s) 2.2 3.5 3.3 D0 queue free % 100 100 100 EM capacity (veh/h) 1416 758 876 Direction, Lane # EB 1 WB 1 WB 2 NB 1 Volume Total 173 1 199 2 Volume Right 0 0 1 SH 1700 1416 1700 813 Volume to Capacity 0.10 0.00 0 Queue Length 95th (ft) 0 0 0 Control Delay (s) 0.0 7.5 0.0 9.4 Approach LOS A A A Approach LOS A A A Approach Cos 0.1 19.6% ICU Level of Service								
vC, conflicting volume 173 374 172 vC1, stage 1 conf vol 172 172 vC2, stage 2 conf vol 201 201 vCu, unblocked vol 173 374 172 C, single (s) 4.1 6.4 6.2 C, 2 stage (s) 5.4 5.4 F (s) 2.2 3.5 3.3 Do queue free % 100 100 100 Do queue free % 100 10 100 100 Dol'ection, Lane # EB 1 WB 1 WB 2 NB 1 Volume Total 173 1 199 2 Volume Right 1 0 1 1 vSH 1700 1416 1700 813 Volume to Capacity 0.10 0.00 0 0 Control Delay (s) 0.0								
vC1, stage 1 conf vol 172 vC2, stage 2 conf vol 201 vCu, unblocked vol 173 374 172 vC, single (s) 4.1 6.4 6.2 vC, 2 stage (s) 5.4 vF (s) 2.2 3.5 3.3 v0 queue free % 100 100 100 cM capacity (veh/h) 1416 758 876 Direction, Lane # EB 1 WB 1 WB 2 NB 1 Volume Total 173 1 199 2 Volume Left 0 1 0 1 volume Right 1 0 0 1 cSH 1700 1416 1700 813 volume to Capacity 0.10 0.00 0.00 Queue Length 95th (ft) 0 0 0 Control Delay (s) 0.0 7.5 0.0 9.4 Ane LOS A A Approach Delay (s) 0.0 0.0 9.4 Approach LOS A A Average Delay 0.1 19.6% ICU Level of Service				173		374	172	
VC2, stage 2 conf vol 201 VCu, unblocked vol 173 374 172 172 173 174 172 174 175 1				175			112	
VCu, unblocked vol 173 374 172 IC, single (s) 4.1 6.4 6.2 IC, 2 stage (s) 5.4 IF (s) 2.2 3.5 3.3 DO queue free % 100 100 100 CM capacity (veh/h) 1416 758 876 Direction, Lane # EB 1 WB 1 WB 2 NB 1 Volume Total 173 1 199 2 Volume Left 0 1 0 1 Volume Right 1 0 0 1 CSH 1700 1416 1700 813 Volume to Capacity 0.10 0.00 0.12 0.00 Queue Length 95th (ft) 0 0 0 0 Control Delay (s) 0.0 7.5 0.0 9.4 Approach Delay (s) 0.0 0.0 9.4 Approach LOS A A Approach LOS A A Approach LOS A A Intersection Summary Average Delay ntersection Capacity Utilization 19.6% ICU Level of Service								
tC, single (s) tC, 2 stage (s) tF (s) 2.2 3.5 3.3 p0 queue free % 100 100 100 cM capacity (veh/h) 1416 758 876 Direction, Lane # EB 1 WB 1 WB 2 NB 1 Volume Total 173 1 199 2 Volume Left 0 1 0 1 1 0 1 1 0 1 0 1 0 1 0 0 1 0				173			172	
## Stage (s) ## St								
#F (s) 2.2 3.5 3.3 p0 queue free % 100 100 100 cM capacity (veh/h) 1416 758 876 Direction, Lane # EB 1 WB 1 WB 2 NB 1 Volume Total 173 1 199 2 Volume Left 0 1 0 1 Volume Right 1 0 0 1 cSH 1700 1416 1700 813 Volume to Capacity 0.10 0.00 0.12 0.00 Queue Length 95th (ft) 0 0 0 0 Control Delay (s) 0.0 7.5 0.0 9.4 Approach Delay (s) 0.0 0.0 9.4 Approach LOS A A Approach LOS A Intersection Summary Average Delay (s) 0.1 ICU Level of Service				4.1			0.2	
100				0.0			2.2	
Direction, Lane # EB 1 WB 1 WB 2 NB 1								
Direction, Lane # EB 1 WB 1 WB 2 NB 1								
Volume Total 173 1 199 2 Volume Left 0 1 0 1 Volume Right 1 0 0 1 SH 1700 1416 1700 813 Volume to Capacity 0.10 0.00 0.12 0.00 Queue Length 95th (ft) 0 0 0 0 Control Delay (s) 0.0 7.5 0.0 9.4 Lane LOS A A A Approach Delay (s) 0.0 0.0 9.4 Approach LOS A A Intersection Summary 0.1 Average Delay 0.1 Intersection Capacity Utilization 19.6% ICU Level of Service	cM capacity (veh/h)			1416		758	8/6	
Volume Left 0 1 0 1 Volume Right 1 0 0 1 SH 1700 1416 1700 813 Volume to Capacity 0.10 0.00 0.12 0.00 Queue Length 95th (ft) 0 0 0 0 Control Delay (s) 0.0 7.5 0.0 9.4 ane LOS A A A approach Delay (s) 0.0 0.0 9.4 approach LOS A A antersection Summary 0.1 ICU Level of Service						- 15		
Volume Right 1 0 0 1 SSH 1700 1416 1700 813 Volume to Capacity 0.10 0.00 0.12 0.00 Queue Length 95th (ft) 0 0 0 0 Control Delay (s) 0.0 7.5 0.0 9.4 Approach Delay (s) 0.0 0.0 9.4 Approach LOS A Approach LOS A Average Delay 0.1 Intersection Capacity Utilization 19.6% ICU Level of Service								
SSH 1700 1416 1700 813 //olume to Capacity 0.10 0.00 0.12 0.00 Queue Length 95th (ft) 0 0 0 0 Control Delay (s) 0.0 7.5 0.0 9.4 Approach Delay (s) 0.0 0.0 9.4 Approach LOS A A Approach LOS A Antersection Summary Average Delay 0.1 Intersection Capacity Utilization 19.6% ICU Level of Service								
Volume to Capacity 0.10 0.00 0.12 0.00 Queue Length 95th (ft) 0 0 0 0 Control Delay (s) 0.0 7.5 0.0 9.4 Approach Delay (s) 0.0 0.0 9.4 Approach LOS A A Approach Summary A A Average Delay 0.1 O.1 Intersection Capacity Utilization 19.6% ICU Level of Service								
Queue Length 95th (ft) 0 0 0 Control Delay (s) 0.0 7.5 0.0 9.4 Lane LOS A A A Approach Delay (s) 0.0 0.0 9.4 Approach LOS A A Average Delay 0.1 ICU Level of Service	SH	1700	1416	1700	813			
Queue Length 95th (ft) 0 0 0 0 Control Delay (s) 0.0 7.5 0.0 9.4 Lane LOS A A Approach Delay (s) 0.0 0.0 9.4 Approach LOS A Intersection Summary A Average Delay ntersection Capacity Utilization 0.1 ICU Level of Service	Volume to Capacity	0.10	0.00	0.12	0.00			
Control Delay (s) 0.0 7.5 0.0 9.4 Lane LOS A A Approach Delay (s) 0.0 0.0 9.4 Approach LOS A Intersection Summary Average Delay 0.1 Intersection Capacity Utilization 19.6% ICU Level of Service		0	0	0	0			
Lane LOS A A Approach Delay (s) 0.0 0.0 9.4 Approach LOS A Intersection Summary A A Average Delay 0.1 ICU Level of Service		0.0	7.5	0.0	9.4			
Approach Delay (s) Output Ou								
Approach LOS A Intersection Summary Average Delay 0.1 Intersection Capacity Utilization 19.6% ICU Level of Service		0.0						
Average Delay 0.1 ntersection Capacity Utilization 19.6% ICU Level of Service								
Average Delay 0.1 ntersection Capacity Utilization 19.6% ICU Level of Service	ntersection Summary							ii x'as xu i xu
stersection Capacity Utilization 19.6% ICU Level of Service				0.1				
		zation			IC	ULevelo	f Service	
	Analysis Period (min)			15.078	.0	2 20 701 0	5517100	

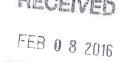
MEGEIVED

	-	•	•	•	4	~
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	1.		7	^	W	21000000
Traffic Volume (veh/h)	162	7	6	178	6	5
Future Volume (Veh/h)	162	7	6	178	6	5
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	176	8	7	193	7	5
Pedestrians		N. F.				
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	TWLTL			TWLTL		
Median storage veh)	2			2		
Upstream signal (ft)	2			2		
pX, platoon unblocked						
vC, conflicting volume			184		387	180
vC1, stage 1 conf vol			104		180	100
vC1, stage 1 conf vol						
			184		207	400
vCu, unblocked vol					387	180
tC, single (s)			4.1		6.4	6.2
tC, 2 stage (s)			0.0		5.4	
tF (s)			2.2		3.5	3.3
p0 queue free %			100		99	99
cM capacity (veh/h)			1403		748	868
Direction, Lane #	EB 1	WB 1	WB 2	NB 1		
Volume Total	184	7	193	12		
Volume Left	0	7	0	7		
Volume Right	8	0	0	5		
cSH	1700	1403	1700	794		
Volume to Capacity	0.11	0.00	0.11	0.02		
Queue Length 95th (ft)	0	0	0	1		
Control Delay (s)	0.0	7.6	0.0	9.6		
Lane LOS		Α		Α		
Approach Delay (s)	0.0	0.3		9.6		
Approach LOS				Α		
Intersection Summary			11 V	tana"		
Average Delay			0.4			
Intersection Capacity Utiliz	zation		19.4%	IC	III evel o	f Service
	LOUUII			IU	O LEVEL C	i Service
Analysis Period (min)			15			

FEB 0 8 2016

	→	•	•	-	1	~	
Movement	EBT	EBR	WBL	WBT	NBL	NBR	N.
Lane Configurations	4		7	†	N.	1000	
Traffic Volume (veh/h)	163	4	4	182	4	3	
Future Volume (Veh/h)	163	4	4	182	4	3	
Sign Control	Free			Free	Stop		
Grade	0%			0%	0%		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	
Hourly flow rate (vph)	177	4	4	198	4	3	
Pedestrians			···		- i		
Lane Width (ft)							
Walking Speed (ft/s)							
Percent Blockage							
Right turn flare (veh)							
Median type	TWLTL			TWLTL			
Median storage veh)	2			2			
Upstream signal (ft)				_			
pX, platoon unblocked							
vC, conflicting volume			181		385	179	
vC1, stage 1 conf vol			101		179	110	
vC2, stage 2 conf vol					206		
vCu, unblocked vol			181		385	179	
tC, single (s)			4.1		6.4	6.2	
tC, 2 stage (s)			4.1		5.4	0.2	
tF (s)			2.2		3.5	3.3	
p0 queue free %			100		99	100	
cM capacity (veh/h)			1407		750	869	
					7 30	609	
Direction, Lane #	EB 1	WB 1	WB 2	NB 1	W 100	N 17	H
Volume Total	181	4	198	7			
Volume Left	0	4	0	4			
Volume Right	4	0	0	3			
cSH	1700	1407	1700	797			
Volume to Capacity	0.11	0.00	0.12	0.01			
Queue Length 95th (ft)	0	0	0	1			
Control Delay (s)	0.0	7.6	0.0	9.6			
Lane LOS		Α		Α			
Approach Delay (s)	0.0	0.1		9.6			
Approach LOS				Α			
Intersection Summary			10,1		-		
Average Delay			0.2			- N	
Intersection Capacity Utiliz	ation		19.6%	IC	U Level o	f Service	,
Analysis Period (min)			15				
maryolo i onou (min)			10				

	-	•	•	←	1	
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	1	- Company	7	1	A	
Traffic Volume (veh/h)	164	2	1	184	2	1
Future Volume (Veh/h)	164	2	1	184	2	1
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	178	2	1	200	2	1
Pedestrians			_			
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	TWLTL			TWLTL		
Median storage veh)	2			2		
Upstream signal (ft)	_			_		
pX, platoon unblocked						
vC, conflicting volume			180		381	179
vC1, stage 1 conf vol			100		179	170
vC2, stage 2 conf vol					202	
vCu, unblocked vol			180		381	179
tC, single (s)			4.1		6.4	6.2
tC, 2 stage (s)			7.1		5.4	0.2
tF (s)			2.2		3.5	3.3
p0 queue free %			100		100	100
cM capacity (veh/h)			1408		754	869
		17100		214.2	704	000
Direction, Lane #	EB 1	WB 1	WB 2	NB 1	- 1	
Volume Total	180	1	200	3		
Volume Left	0	1	0	2		
Volume Right	2	0	0	1		
cSH	1700	1408	1700	789		
Volume to Capacity	0.11	0.00	0.12	0.00		
Queue Length 95th (ft)	0	0	0	0		
Control Delay (s)	0.0	7.6	0.0	9.6		
Lane LOS		Α		Α		
Approach Delay (s)	0.0	0.0		9.6		
Approach LOS				Α		
Intersection Summary	J					
Average Delay			0.1			
Intersection Capacity Utiliz	ation		19.7%	IC	U Level o	f Service
Analysis Period (min)			15	10		. 30, 1,00



KITTELSON & ASSOCIATES, INC.

610 SW Alder, Suite 700 Portland, Oregon 97205 (503) 228-5230 Fax: (503) 273-8169

Project #: Project Name: 19056

SOZO Sports

Analyst: Date:

BHR 2/4/2016

Flle:

Intersection:

Ahtanum Road/S 38th Ave

Scenario:

Phase 3

Warrant Summary

Warrant	Name	Analyzed?	Met?
#1	Eight-Hour Vehicular Volume	Yes	Yes
#2	Four-Hour Vehicular volume	Yes	Yes
#3	Peak Hour	Yes	Yes*
#4	Pedestrian Volume	No	98
#5	School Crossing	No	
#6	Coordinated Signal System	No	2
#7	Crash Experience	No	-
#8	Roadway Network	No	

Input Parameters

Volume Adjustment Factor ≈	1.0
North-South Approach =	minor
East-West Approach =	Major
Major Street Thru Lanes =	1
Minor Street Thru Lanes =	1
Speed > 40 mph?	Yes
Population < 10,000?	No
Warrant Factor	70%
Peak Hour or Daily Count?	Daily

Analysis Traffic Volumes

Hour		Major	Street	Minor Street		
Begin	End	EB	WB	NB	SB	
12:00 AM	1:00 AM	10	27	0	3	
1:00 AM	2:00 AM	7	14	0	2	
2:00 AM	3:00 AM	11	27	0	3	
3:00 AM	4:00 AM	25	17	0	2	
4:00 AM	5:00 AM	77	128	0	17	
5:00 AM	6:00 AM	170	214	0	35	
6:00 AM	7:00 AM	381	201	0	57	
7:00 AM	8:00 AM	553	224	0	87	
8:00 AM	9:00 AM	363	230	0	65	
9:00 AM	10:00 AM	279	247	0	60	
10:00 AM	11:00 AM	250	287	0	67	
11:00 AM	12:00 PM	246	314	0	74	
12:00 PM	1:00 PM	253	325	0	70	
1:00 PM	2:00 PM	252	329	0	48	
2:00 PM	3:00 PM	322	426	0	66	
3:00 PM	4:00 PM	330	546	0	94	
4:00 PM	5:00 PM	341	600	0	95	
5:00 PM	6:00 PM	338	579	0	99	
6:00 PM	7:00 PM	199	386	0	51	
7:00 PM	8:00 PM	178	230	0	51	
8:00 PM	9:00 PM	121	216	0	30	
9:00 PM	10:00 PM	111	202	0	28	
10:00 PM	11:00 PM	73	95	0	18	
11:00 PM	12:00 AM	29	67	0	9	

Warrant	#1	- Claht	Marie

Warrant Factor	Condition	Major Street Requirement	Minor Street Requirement	Hours That Condition Is Met	Condition for Warrant Factor Met?	Signal Warrant Met?	
100%	А	500	150	0	No	No	
100%	В	750	75	4	No	140	
900/	Α	400	120	0	No	Ne	
80%	В 60	600	60	5	No	No	
700/	Α	350	105	0	No	W	
70%	В	525	53	11	Yes	Yes	

RECEIVED

DEPARTMENT OF PUBLIC SERVICES



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VERN M. REDIFER, P.E., Director

DEC 17 2015
CITY OF YAKIMA
PLANNING DIV

December 17, 2015

Joan Davenport, AICP City of Yakima 129 N. 2nd Street Yakima, Washington 98901

Dear Mrs. Davenport:

SUBJECT: Review of the Draft MDNS and Traffic Study for the SOZO Sports Multi-Sport Complex

Yakima County has reviewed that draft Mitigated Determination of Non-significance (MDNS) and have the following comments:

- 1. The MDNS should address all impacts to the natural and built environments, including construction of onsite and offsite improvements required for the project. This includes, but is not limited to, impacts to the floodplain/floodway from road improvements, traffic impacts on nearby local access roads, etc.
- 2. Under the Findings #7 of the MDNS it states that, "all proposed road and utilities for development of Phase 1 are located outside of the 100-year floodplain, and any critical areas," however what about additional phases?
- 3. Page 3 of the MDNS states that Yakima County has jurisdiction over the three intersections on Ahtanum Rd. within the study area. Yakima County has jurisdiction over the intersections at S. 38th and S.52nd, but not the intersection of S.16th.
- 4. The Traffic Impact Analysis (TIA) has several required revisions:
 - a. The TIA states that the site is located in a "predominately rural area". The site is actually located within the City of Yakima and is not in a rural area or setting. As such the proposed project should provide provisions for other modes of transportation including walkers/joggers and bicycles. There is no analysis of alternative modes of travel in the document.
 - b. The TIA refers to two access points into the site, however the site plans shows three access points. The trips distribution should be re-evaluated based on the three access points.
 - c. The study did not address the intersections that are immediately adjacent to the site. S. 38th @ Sorensen and Sorenson @ S.36th for operation and delay due to the proposed alignment revisions and additional traffic volumes.

- d. The TIA did not address additional delays and impacts to Springcreek Rd. @ S.40th, Springcreek Rd. @ S.41st, Springcreek Rd @ S. 44th, Springcreek Rd @ S. 45th, Springcreek Rd @ S. 47th, S. 38th @ W. Larch Ave and S. 38th @ Oak Ave. for impacts and delays to the existing residences.
- e. The TIA did not provide any roadway capacity analysis, only intersection capacity was provided. The existing configuration of S. 36th, S.38th and Sorensen may not be able to handle the additional trips without improvements.
- f. S. 38th @ Ahtanum was identified in the TIA as requiring signalization. Yakima County does not believe that signalization of this intersection provides any benefit to the overall transportation system. It will add additional delay to traffic using Ahtanum Road in favor of the traffic accessing the new sports complex. Yakima County requested that the site be accessed by an extension of Occidental Road from S. 52nd to S.38th. This connection was not addressed in the TIA. The signalization of S.38th @ Ahtanum Road may be considered if the signal is temporary (installed on spanwire) and with the understanding that when the extension of Occidental is completed that the signal hardware will be relocated to the intersection of S.52nd and Ahtanum. The required widening of Ahtanum Road to accommodate the left turn lane at S.38th will be required to be designed and constructed to Yakima County standards by the project proponents.
- 5. Section B, Transportation will be required to be modified to reflect the revision to the TIA.
- 6. The phasing of the project will need to be revisited based the fact that the TIA assumed that there were no roadway capacity issues on the roadways adjacent to the site. However, it is evident that S.36th and Sorensen cannot accommodate that increased traffic and that improvements will be required. Because of the required construction on these roadways the only access with be via S.36th. This will require revisions to the phasing plan and the timing of the proposed improvements.

7. The conclusions will need to be modified to address the above revisions.

If you have any questions please let us know.

Sincerely

Gary N. Ekstedt, P.E.

County Engineer

Lynn Deitrick, AICP Planning Official



Yakima County ensures full compliance with Title VI of the Civil Rights Act of 1964 by prohibiting discrimination against any person on the basis of race, color, nation origin, or sex in the provision of benefits and services resulting from its federally assisted programs and activities. For questions regarding Yakima County's Title VI Program, you may contact the Title VI Coordinator at 509-574-2300.

If this letter pertains to a meeting and you need special accommodations, please call us at 509-574-2300 by 10:00 a.m. three days prior to the meeting. For TDD users, please use the State's toil free relay service 1-800-833-6388 and ask the operator to dial 509-574-2300.



Public Services

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VERN M. REDIFER, P.E. - Director

February 23, 2016

Mr. Matt Hughart, AICP Kittelson & Associates, Inc. 610 SW Alder Street Suite 700 Portland, Oregon 97205

Dear Mr. Hughart!

SUBJECT:

SOZO Sports of Central Washington

Transportation Impact Analysis - Supplemental Analysis

We have reviewed your supplemental analysis for the above project and have the following comments:

- 1. The supplemental analysis talks about the operation of eight soccer fields by spring of 2016. Neither the report nor the associated maps clearly identifies the eight field that will be operated in phase 1 of construction. Please clarify the location of the eight soccer fields that will be open and in operation in the spring of 2016.
- 2. Delay to roadways intersecting Spring Creek Road. Yakima County had requested additional analysis to determine the impacts to the residences accessing Spring Creek Road from S. 40th Avenue, S. 41st Avenue, S. 44 Avenue, S. 45th Avenue and S 47th Avenue. You provided Level-of-Service analysis for these intersections, but what was requested was the amount (or percent) of increased delay at these intersections. Please provide the amount of increased delay at these intersections.
- 3. The intersection of S. 38th Avenue and Sorenson Road has been analyzed as a yield condition. Yakima County will not operate this intersection as a yield. The yield does not adequately assign the right-of-way to all movements in the intersection. This intersection will need to be reanalyzed with stop control.
- 4. The intersection of S. 36th Avenue and Sorenson Road has been analyzed as a continuous movement. Yakima County has previously indicated that the geometry is not adequate for this to operate in this configuration. The recommendation was made to make this a "T" intersection to accommodate the lack of curve radius. Either this intersection will need to be re-analyzed as a "T" intersection or additional right-of-way will need to be dedicated in order to construct curves that meet AASHTO standards for a 25 MPH design speed.

Yakima County ensures full compliance with Title VI of the Civil Rights Act of 1964 by prohibiting discrimination against any person on the basis of race, color, national origin, or sex in the provision of benefits and services resulting from its federally assisted programs and activities. For questions regarding Yakima County's Title VI Program, you may contact the Title VI Coordinator at 509-574-2300.

If this letter pertains to a meeting and you need special accommodations, please call us at 509-574-2300 by 10:00 a.m. three days prior to the meeting. For TDD users, please use the State's toll free relay service 1-800-833-6388 and ask the operator to dial 509-574-2300.

5. It appears that the intersection of Ahtanum and S. 38th Avenue will require signalization for the operation of the eight fields during the construction of S. 36th Avenue. Given the fact that The City of Yakima is scheduled to go to construction of S. 36th Avenue in 2016 and that signal plans have been prepared or approved. Please identify how many fields can be in operation with no traffic signal and S. 36th Avenue under construction.

Please let me know if you have any questions or require any clarification.

Sincerely,

Kent L. McHenry, P.E.

Transportation Engineering Manager

Peters, Jeff

From: Matt Hughart <MHUGHART@kittelson.com>

Sent: Wednesday, March 02, 2016 7:01 AM

To: Davenport, Joan; Sheffield, Brett; Peters, Jeff; Leanne Liddicoat

Subject: Fwd: Updated Yakima Tables

Joan,

Here are a revised set of Tables 2 and 3 from our response memo that addresses the change in delay along the Spring Creek Road intersections. Please review and forward to Kent if you have no further questions. Per our discussion on Monday, we will not prepare responses to the other questions in Kent's e-mail. Please let me know if you need anything else.

Matt Hughart, AICP Kittelson & Associates, Inc. 503-228-5230

Begin forwarded message:

From: Zachary Bugg <zbugg@kittelson.com>
Date: March 2, 2016 at 6:51:24 AM PST

To: Matt Hughart < MHUGHART@kittelson.com >

Subject: Updated Yakima Tables

Table 2 - Spring Creek Road Delay Analysis - Partial 8-Field Build-out

		Spring Creek / 40th Spr		Spring Creek / 41st		Spring Creek / 44th		Spring Creek / 45th		Spring Creek / 47	
Number of Houses Served			6	16		2	25		4	23	
				Exist	ing Conditio	ens					
Time Period PM SAT			PM	SAT	PM	SAT	PM	SAT	PM	SAT	
Northbound Approach LOS A		А	A	Α	Α	Α	Α	Α	Α	Α	Α
Northbound Approach Delay (sec)		8.8	8.7	8.9	8.7	8.9	8.7	9.1	8.6	8.9	8.6
				Partial	8-Field Build	d-out					
Tria Commention	In	4	3	10	8	16	13	3	2	14	12
Trip Generation	Out	2	3	6	7	9	11	1	2	9	10
Northbound LOS		А	Α	Α	Α	Α	Α	Α	Α	Α	Α
Northbound Approach Delay (sec)		9.3	9.3	9.5	9.2	9.4	9.3	9.8	9.1	9.5	9.2

Table 3 - Spring Creek Road Delay Analysis – Full Site Build-out

	Spring Creek / 40th		Spring Creek / 41st		Spring Creek / 44th		Spring Creek / 45th		Spring Creek / 47th	
Number of Houses Served	6		16 25		4		23			
Existing Conditions										
Time Period	PM	SAT								
Northbound Approach LOS	Α	Α	Α	Α	Α	Α	А	Α	Α	Α
Northbound Approach Delay (sec)	8.8	8.7	8.9	8.7	8.9	8.7	9.1	8.6	8.9	8.6

Full Site Build-out											
Trip Generation Out	In	4	3	10	8	16	13	3	2	14	12
	Out	2	3	6	7	9	11	1	2	9	10
Northbound LOS		Α	Α	А	Α	Α	Α	Α	Α	Α	Α
Northbound Approach Delay (sec)		9.4	9.6	9.5	9.6	9.5	9.6	9.8	9.4	9.5	9.5

Zachary Bugg, PhD, El Engineering Associate

Kittelson & Associates, Inc.
Transportation Engineering / Planning
36 South Charles Street, Suite 1920
Baltimore, Maryland 21201
410.347.9610
443.524.9413 (direct)

Streetwise Twitter Facebook

<image001.jpg>

Peters, Jeff

From: Davenport, Joan

Sent: Thursday, February 25, 2016 4:28 PM

To: Matt Hughart; Kent McHenry (kent.mchenry@co.yakima.wa.us); Sheffield, Brett; Peters,

Jeff

Cc: Leanne Liddicoat

Subject: City of Yakima response to Yakima County traffic comments

These remarks are in response to the letter from Kent McHenry, dated February 23, 2016 related to comments from the Kittelson Associates Supplemental Traffic Analysis for the SOZO Sports Complex. I do not see that any of Kent's questions would require additional analysis by Matt – I am providing my responses. Matt and others, please feel free to weigh in. Jeff Peters and I are waiting to hear from the group to see if we are "good to go" on the analysis.

- 1. Supplemental Traffic Analysis does not identify location of up to 8 fields in Phase 1: An updated site plan will be provided.
- 2. Delay to side streets & intersections with Spring Creek Road Table 2 of the supplemental report lists by intersection the number of added trips to each of the side streets (40th Ave, 41st Ave, 44th Ave, 45th Ave and 47th Ave). The report concludes that all streets will continue to operate at Level of Service "A". Appendix A included the detail work sheets.
- 3. Concern about YIELD control at 38th Ave & Ahtanum Rd. The proposed SEPA MDNS includes the requirement for construction of a "T" intersection design at 38th Ave and Sorenson Road. Installation of STOP control is appropriate. No additional analysis is requested from Kittelson Associates. Yakima County Public Works will dictate the appropriate design, and right of way for street construction, which the City will install with the support of SIED funds.
- 4. The intersection of 36th Avenue and Sorenson Road *will be constructed to Yakima County specifications. No additional traffic analysis is requested from Kittelson Associates.*
- 5. How many fields can be played without a traffic signal at 38th Ave & Ahtanum? The Traffic Analysis states "Based on an iterations analysis, it was concluded that signalization and associated widening of Ahtanum would be needed at the equivalent of 12 or more soccer fields.

Joan Davenport, AICP
Director of Community Development
City of Yakima
129 North 2nd St
Yakima, WA 98901
Joan.davenport@yakimawa.gov
(509)576-6417